REVISION RECORD FOR THE
STATE OF CALIFORNIA

ERRATA

September 1, 2017

2016 Title 24, Part 2.5, California Residential Code

General Information:

1. The date of these errata is for identification purposes only. See the History Note Appendix at the end of the code.

2. These errata are issued by the California Building Standards Commission in order to correct nonsubstantive printing errors or omissions in California Code of Regulations, Title 24, Part 2.5, of the 2016 California Residential Code. Instructions are provided below.

3. Health and Safety Code Section 18938.5 establishes that only building standards in effect at the time of the application for a building permit may be applied to the project plans and construction. This rule applies to both adoptions of building standards for Title 24 by the California Building Standards Commission and local adoptions and ordinances imposing building standards. An erratum to Title 24 is a nonregulatory correction because of a printing error or omission that does not differ substantively from the official adoption by the California Building Standards Commission. Accordingly, the corrected code text provided by this erratum may be applied on and after the stated effective date.

4. You may wish to retain the superseded material with this revision record so that the prior wording of any section can be easily ascertained.

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Item No. 5525S162
Part II—Definitions

CHAPTER 2
DEFINITIONS

Code change proposals to definitions in this chapter preceded by a bracketed letter are considered by the IRC-Building Code Development Committee [RB] or the IECC-Residential Code Development Committee [RE] during the Group B (2016) Code Development cycle. See page x for explanation.

SECTION R201
GENERAL

R201.1 Scope. Unless otherwise expressly stated, the following words and terms shall, for the purposes of this code, have the meanings indicated in this chapter.

R201.2 Interchangeability. Words used in the present tense include the future; words in the masculine gender include the feminine and neuter; the singular number includes the plural and the plural, the singular.

R201.3 Terms defined in other codes. Where terms are not defined in this code such terms shall have meanings ascribed to them as in the California Building Standards Code, Title 24, California Code of Regulations.

R201.4 Terms not defined. Where terms are not defined through the methods authorized by this section, such terms shall have ordinarily accepted meanings such as the context implies.

For applications listed in Section 1.11 regulated by the Office of the State Fire Marshal, where terms are not defined through the methods authorized by this section, such terms shall have ordinarily accepted meanings such as the context implies. Webster’s Third New International Dictionary of the English Language, Unabridged, shall be considered as providing ordinarily accepted meanings.

SECTION R202
DEFINITIONS

[RB] ACCESSORY STRUCTURE. A structure that is accessory to and incidental to that of the dwelling(s) and that is located on the same lot.

[RB] ADDITION. An extension or increase in floor area or height of a building or structure.

[RB] ADHERED STONE OR MASONRY VENEER. Stone or masonry veneer secured and supported through the adhesion of an approved bonding material applied to an approved backing.

AGED HOME OR INSTITUTION. A facility used for the housing of persons 65 years of age or older in need of care and supervision. (See definition of “care and supervision.”)

[RB] AIR-IMPERMEABLE INSULATION. An insulation having an air permanence equal to or less than 0.02 L/s-m² at 75 Pa pressure differential as tested in accordance with ASTM E2178 or E283.

[RB] ALTERATION. Any construction or renovation to an existing structure other than repair or addition.

[RB] ALTERNATING TREAD DEVICE. A device that has a series of steps between 50 and 70 degrees (0.87 and 1.22 rad) from horizontal, usually attached to a center support rail in an alternating manner so that the user does not have both feet on the same level at the same time.

[RB] ANCHORED STONE OR MASONRY VENEER. Stone or masonry veneer secured with approved mechanical fasteners to an approved backing.

ANCHORS. See “Supports.”

[RB] APPROVED. Acceptable to the building official.

APPROVED. (HCD 1) Meeting the approval of the enforcing agency, except as otherwise provided by law, when used in connection with any system, material, type of construction, fixture or appliance as the result of investigations and tests conducted by the agency, or by reason of accepted principles or tests by national authorities or technical, health, or scientific organizations or agencies.

Notes:

1. See Health and Safety Code Section 17920 for “Approved” as applied to residential construction and buildings or structures accessory thereto, as referenced in Section 1.8.2.1.1.

2. See Health and Safety Code Section 17921.1 for “Approved” as applied to the use of hotplates in residential construction referenced in Section 1.8.2.1.1.

3. See Health and Safety Code Section 19966 for “Approved” as applied to factory-built housing as referenced in Section 1.8.3.2.5.

4. See Health and Safety Code Section 18201 for “Approved” as applied to mobilehome parks as referenced in Section 1.8.3.2.2.

5. See Health and Safety Code Section 18862.1 for “Approved” as applied to special occupancy parks as referenced in Section 1.8.3.2.3.

[RB] APPROVED AGENCY. An established and recognized agency regularly engaged in conducting tests or furnishing inspection services, where such agency has been
DEFINITIONS

approved by the building official. (HCD 1) “Approved agency” shall mean “Listing agency” and “Testing agency.”

APPROVED LISTING AGENCY. Any agency approved by the enforcing agency, unless otherwise provided by statute, which is in the business of listing and labeling and which makes available at least an annual published report of such listings in which specific information is included that the product has been tested to recognized standards and found to comply.

APPROVED TESTING AGENCY. Any agency which is determined by the enforcing agency, except as otherwise provided by statute, to have adequate personnel and expertise to carry out the testing of systems, materials, and construction fixtures or appliances.

[RB] ASPECT RATIO. The ratio of longest to shortest perpendicular dimensions, or for wall sections, the ratio of height to length.

[RB] ATTIC. The unfinished space between the ceiling assembly and the roof assembly.

[RB] ATTIC, HABITABLE. A finished or unfinished area, not considered a story, complying with all of the following requirements:

1. The occupiable floor area is not less than 70 square feet (6.5 m²), in accordance with Section R304.

2. The occupiable floor area has a ceiling height in accordance with Section R305.

3. The occupiable space is enclosed by the roof assembly above, knee walls (if applicable) on the sides and the floor-ceiling assembly below.

[RB] BASEMENT. A story that is not a story above grade plane. (see “Story above grade plane”).

[RB] BASEMENT WALL. The opaque portion of a wall that encloses one side of a basement and has an average below grade wall area that is 50 percent or more of the total opaque and nonopaque area of that enclosing side.

[RB] BASIC WIND SPEED. Three-second gust speed at 33 feet (10.058 mm) above the ground in Exposure C (see Section R301.2.1) as given in Figure R301.2(4)A.

BEDRIDDEN PERSON. A person, requiring assistance in turning and repositioning in bed, or being unable to independently transfer to and from bed, except in facilities with appropriate and sufficient care staff, mechanical devices if necessary, and safety precautions as determined in Title 22 regulations, by the Director of Social Services or his or her designated representative. Persons who are unable to independently transfer to and from bed, but who do not need assistance to turn or reposition in bed, shall be considered nonambulatory.

The Director of Social Services or his or her designated representative shall make the determination of the bedridden status of persons with developmental disabilities, in consultation with the Director of Developmental Services or his or her designated representative.

The Director of Social Services or his or her designated representative shall make the determination of the bedridden status of all other persons with disabilities who are not developmentally disabled.

[RB] BOND BEAM. A horizontal grouted element within masonry in which reinforcement is embedded.

[RB] BRACED WALL LINE. A straight line through the building plan that represents the location of the lateral resistance provided by the wall bracing.

[RB] BRACED WALL LINE, CONTINUOUSLY SHEATHED. A braced wall line with structural sheathing applied to all sheathable surfaces including the areas above and below openings.

[RB] BRACED WALL PANEL. A full-height section of wall constructed to resist in-plane shear loads through interaction of framing members, sheathing material and anchors. The panel’s length meets the requirements of its particular bracing method, and contributes toward the total amount of bracing required along its braced wall line in accordance with Section R602.10.1.

[RB] BUILDING. Building shall mean any one- and two-family dwelling or portion thereof, including townhouses, that is used, or designed or intended to be used for human habitation, for living, sleeping, cooking or eating purposes, or any combination thereof, and shall include accessory structures thereto.

Exceptions: For applications listed in Section 1.8.2 regulated by the Department of Housing and Community Development, “Building” shall not include the following:

1. Any mobilehome as defined in Health and Safety Code Section 18008.

2. Any manufactured home as defined in Health and Safety Code Section 18007.

3. Any commercial modular as defined in Health and Safety Code Section 18001.8 or any special purpose commercial modular as defined in Section 18012.5.

4. Any recreational vehicle as defined in Health and Safety Code Section 18010.

5. Any multifamily manufactured home as defined in Health and Safety Code Section 18008.7.

For additional information, see Health and Safety Code Section 18908.

Note: Building shall have the same meaning as defined in Health and Safety Code Sections 17920 and 18908 for the applications specified in Section 1.11.

[RB] BUILDING, EXISTING. Existing building is a building erected prior to the adoption of this code, or one for which a legal building permit has been issued.

[RB] BUILDING-INTEGRATED PHOTOVOLTAIC PRODUCT. A building product that incorporates photovoltaic modules and functions as a component of the building envelope.

[RB] BUILDING LINE. The line established by law, beyond which a building shall not extend, except as specifically provided by law.
R301.2.1 Wind design criteria. Buildings and portions thereof shall be constructed in accordance with the wind provisions of this code using the ultimate design wind speed in Table R301.2(1) as determined from Figure R301.2(4)A. The structural provisions of this code for wind loads are not permitted where wind design is required as specified in Section R301.2.1.1. Where different construction methods and structural materials are used for various portions of a building, the applicable requirements of this section for each portion shall apply. Where not otherwise specified, the wind loads listed in Table R301.2(2) adjusted for height and exposure using Table R301.2(3) shall be used to determine design load performance requirements for wall coverings, curtain walls, roof coverings, exterior windows, skylights, garage doors and exterior doors. Asphalt shingles shall be designed for wind speeds in accordance with Section R905.2.4. A continuous load path shall be provided to transmit the applicable uplift forces in Section R802.11.1 from the roof assembly to the foundation.

R301.2.1.1 Wind limitations and wind design required. The wind provisions of this code shall not apply to the design of buildings where wind design is required in accordance with Figure R301.2(4)B.

Exceptions:

1. For concrete construction, the wind provisions of this code shall apply in accordance with the limitations of Sections R404 and R608.

2. For structural insulated panels, the wind provisions of this code shall apply in accordance with the limitations of Section R610.

3. For cold-formed steel light-frame construction, the wind provisions of this code shall apply in accordance with the limitations of Sections R505, R603 and R804.

In regions where wind design is required in accordance with Figure R301.2(4)B, the design of buildings for wind loads shall be in accordance with one or more of the following methods:

1. AIFI Wood Frame Construction Manual (WFCM).
2. ICC Standard for Residential Construction in High-Wind Regions (ICC 600).
4. AISI Standard for Cold-Formed Steel Framing—Prescriptive Method For One- and Two-Family Dwellings (AISI S230).

The elements of design not addressed by the methods in Items 1 through 5 shall be in accordance with the provisions of this code.

Where ASCE 7 or the California Building Code is used for the design of the building, the wind speed map and exposure category requirements as specified in ASCE 7 and the California Building Code shall be used.

R301.2.1.1.1 Sunrooms. Sunrooms shall comply with AAMA/NPEA/NSA 2100. For the purpose of applying the criteria of AAMA/NPEA/NSA 2100
based on the intended use, sunrooms shall be identified as one of the following categories by the permit applicant, design professional or the property owner or owner’s agent in the construction documents. Component and cladding pressures shall be used for the design of elements that do not qualify as main windforce-resisting systems. Main windforce-resisting system pressures shall be used for the design of elements assigned to provide support and stability for the overall sunroom.

Category I: A thermally isolated sunroom with walls that are open or enclosed with insect screening or 0.5 mm (20 mil) maximum thickness plastic film. The space is nonhabitable and unconditioned.

Category II: A thermally isolated sunroom with enclosed walls. The openings are enclosed with translucent or transparent plastic or glass. The space is nonhabitable and unconditioned.

Category III: A thermally isolated sunroom with enclosed walls. The openings are enclosed with translucent or transparent plastic or glass. The sunroom fenestration complies with additional requirements for air infiltration resistance and water penetration resistance. The space is nonhabitable and unconditioned.

Category IV: A thermally isolated sunroom with enclosed walls. The sunroom is designed to be heated or cooled by a separate temperature control system and is thermally isolated from the primary structure. The sunroom fenestration complies with additional requirements for water penetration resistance, air infiltration resistance and thermal performance. The space is nonhabitable and conditioned.

Category V: A sunroom with enclosed walls. The sunroom is designed to be heated or cooled and is open to the main structure. The sunroom fenestration complies with additional requirements for water penetration resistance, air infiltration resistance and thermal performance. The space is habitable and conditioned.

R301.2.1.2 Protection of openings. Exterior glazing in buildings located in windborne debris regions shall be protected from windborne debris. Glazed opening protection for windborne debris shall meet the requirements of the Large Missile Test of ASTM E1996 and ASTM E1886 as modified in Section 301.2.1.2.1. Garage door glazed opening protection for windborne debris shall meet the requirements of an approved impact-resisting standard or ANSI/DASMA 115.

Exception: Wood structural panels with a thickness of not less than 7/16 inch (11 mm) and a span of not more than 8 feet (2438 mm) shall be permitted for opening protection. Panels shall be precut and attached to the framing surrounding the opening containing the product with the glazed opening. Panels shall be predrilled as required for the anchorage method and shall be secured with the attachment hardware provided. Attachments shall be designed to resist the component and cladding loads determined in accordance with either Table R301.2.2 or ASCE 7, with the permanent corrosion-resistant attachment hardware provided and anchors permanently installed on the building. Attachment in accordance with Table R301.2.1.2 is permitted for buildings with a mean roof height of 45 feet (13,728 mm) or less where the ultimate design wind speed, $V_{sw}$ is 180 mph (290 kph) or less.

**TABLE R301.2.1.2**

<table>
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<tr>
<th>FASTENER TYPE</th>
<th>FASTENER SPACING (inches)</th>
<th>Panel span ≤ 4 feet</th>
<th>4 feet &lt; panel span ≤ 6 feet</th>
<th>6 feet &lt; panel span ≤ 8 feet</th>
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<td>7/16-inch lag screw based anchor with 2-inch embedment length</td>
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</table>

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound = 4.448 N, 1 mile per hour = 0.447 m/s.

- a. This table is based on 180 mph ultimate design wind speeds, $V_{sw}$, and a 45-foot mean roof height.
- b. Fasteners shall be installed at opposing ends of the wood structural panel.
- c. Anchors shall penetrate through the exterior wall covering with an embedment length of not less than 2 inches into the building frame. Fasteners shall be located not less than 2/3 inches from the edge of concrete block or concrete.
- d. Panels attached to masonry or masonry/stucco shall be attached using vibration-resistant anchors having an ultimate withdrawal capacity of not less than 1,500 pounds.

R301.2.1.2.1 Application of ASTM E1996. The text of Section 2.2 of ASTM E1996 shall be substituted as follows:

2.2 ASCE Standard:

ASCE 7-10 American Society of Civil Engineers Minimum Design Loads for Buildings and Other Structures

The text of Section 6.2.2 of ASTM E1996 shall be substituted as follows:

6.2.2 Unless otherwise specified, select the wind zone based on the ultimate design wind speed, $V_{sw}$, as follows:

- 6.2.2.1 Wind Zone 1 – 130 mph ≤ ultimate design wind speed, $V_{sw}$ < 140 mph.
- 6.2.2.2 Wind Zone 2 – 140 mph ≤ ultimate design wind speed, $V_{sw}$ < 150 mph at greater than 1 mile (1.6 km) from coastline. The coastline shall be measured from the mean high water mark.
mm) from the edge of the opening. This provision is applicable to shear walls and braced wall panels offset in plane and to braced wall panels offset out of plane as permitted by the exception to Item 1.

**Exception:** For wood light-frame wall construction, one end of a braced wall panel shall be permitted to extend more than 1 foot (305 mm) over an opening not more than 8 feet (2438 mm) in width in the wall below provided that the opening includes a header in accordance with the following:

1. The building width, loading condition and framing member species limitations of Table R602.7(1) shall apply; and
2. Not less than one 2 x 12 or two 2 x 10 for an opening not more than 4 feet (1219 mm) wide; or
3. Not less than two 2 x 12 or three 2 x 10 for an opening not more than 6 feet (1829 mm) in width; or
4. Not less than three 2 x 12 or four 2 x 10 for an opening not more than 8 feet (2438 mm) in width; and
5. The entire length of the braced wall panel does not occur over an opening in the wall below.

4. Where an opening in a floor or roof exceeds the lesser of 12 feet (3658 mm) or 50 percent of the least floor or roof dimension.

5. Where portions of a floor level are vertically offset.

**Exceptions:**

1. Framing supported directly by continuous foundations at the perimeter of the building.
2. For wood light-frame construction, floors shall be permitted to be vertically offset when the floor framing is lapped or tied together as required by Section R502.6.1.

6. Where shear walls and braced wall lines do not occur in two perpendicular directions.

7. Where stories above grade plane partially or completely braced by wood wall framing in accordance with Section R602 or cold-formed steel wall framing in accordance with Section R603 include masonry or concrete construction. Where this irregularity applies, the entire story shall be designed in accordance with accepted engineering practice.

**Exception:** Fireplaces, chimneys and masonry veneer as permitted by this code.

**R301.2.2.3 Seismic Design Categories D<sub>p</sub>, D<sub>1</sub>, and D<sub>2</sub>**

Structures assigned to Seismic Design Categories D<sub>p</sub>, D<sub>1</sub>, and D<sub>2</sub> shall conform to the requirements for Seismic Design Category C and the additional requirements of this section.

**R301.2.2.3.1 Height limitations.** Wood-framed buildings shall be limited to three stories above grade plane or the limits given in Table R602.10.3(3). Cold-formed, steel-framed buildings shall be limited to less than or equal to three stories above grade plane in accordance with AISI S230. Mezzanines as defined in Section R202 that comply with Section R325 shall not be considered as stories. Structural insulated panel buildings shall be limited to two stories above grade plane.

**R301.2.2.3.2 Stone and masonry veneer.** Anchored stone and masonry veneer shall comply with the requirements of Sections R702.1 and R703.

**R301.2.2.3.3 Masonry construction.** Masonry construction in Seismic Design Categories D<sub>p</sub>, D<sub>1</sub>, and D<sub>2</sub> shall comply with the requirements of Section R606.12.1. Masonry construction in Seismic Design Category D<sub>2</sub> shall comply with the requirements of Section R606.12.4.

**R301.2.2.3.4 Concrete construction.** Buildings with exterior above-grade concrete walls shall comply with PCA 100 or shall be designed in accordance with ACI 318.

**R301.2.2.3.5 Cold-formed steel framing in Seismic Design Categories D<sub>p</sub>, D<sub>1</sub>, and D<sub>2</sub>.** In Seismic Design Categories D<sub>p</sub>, D<sub>1</sub>, and D<sub>2</sub> in addition to the requirements of this code, cold-formed steel framing shall comply with the requirements of AISI S230.

**R301.2.2.3.6 Masonry chimneys.** Masonry chimneys shall be reinforced and anchored to the building in accordance with Sections R1003.3 and R1003.4.

**R301.2.2.3.7 Anchorage of water heaters.** Water heaters shall be anchored against movement and overturning in accordance with the California Plumbing Code.

**R301.2.2.4 Seismic Design Category E.** Buildings in Seismic Design Category E shall be designed to resist seismic loads in accordance with the California Building Code, except where the seismic design category is reclassified to a lower seismic design category in accordance with Section R301.2.2.1. Components of buildings not required to be designed to resist seismic loads shall be constructed in accordance with the provisions of this code.

**R301.2.3 Snow loads.** Wood-framed construction, cold-formed, steel-framed construction and masonry and concrete construction, and structural insulated panel construction in regions with ground snow loads 70 pounds per square foot (3.35 kPa) or less, shall be in accordance with Chapters 5, 6 and 8. Buildings in regions with ground snow loads greater than 70 pounds per square foot (3.35 kPa) shall be designed in accordance with accepted engineering practice.
R301.2.4 Floodplain construction. Buildings and structures constructed in whole or in part in flood hazard areas (including A or V Zones) as established in Table R301.2(1), and substantial improvement and restoration of substantial damage of buildings and structures in flood hazard areas, shall be designed and constructed in accordance with Section R322. Buildings and structures that are located in more than one flood hazard area shall comply with the provisions associated with the most restrictive flood hazard area. Buildings and structures located in whole or in part in identified floodways shall be designed and constructed in accordance with ASCE 24.

R301.2.4.1 Alternative provisions. As an alternative to the requirements in Section R322, ASCE 24 is permitted subject to the limitations of this code and the limitations therein.

R301.3 Story height. The wind and seismic provisions of this code shall apply to buildings with story heights not exceeding the following:

1. For wood wall framing, the story height shall not exceed 11 feet 7 inches (3531 mm) and the laterally unsupported bearing wall stud height permitted by Table R602.3(5).

2. For cold-formed steel wall framing, the story height shall be not more than 11 feet 7 inches (3531 mm) and the unsupported bearing wall stud height shall be not more than 10 feet (3048 mm).

3. For masonry walls, the story height shall not be more than 13 feet 7 inches (4140 mm) and the bearing wall clear height shall be not greater than 12 feet (3658 mm).

   Exception: An additional 8 feet (2438 mm) of bearing wall clear height is permitted for gable end walls.

4. For insulating concrete form walls, the maximum story height shall not exceed 11 feet 7 inches (3531 mm) and the maximum unsupported wall height per story as permitted by Section R608 tables shall not exceed 10 feet (3048 mm).

5. For structural insulated panel (SIP) walls, the story height shall be not greater than 11 feet 7 inches (3531 mm) and the bearing wall height per story as permitted by Section R610 tables shall not exceed 10 feet (3048 mm).

Individual walls or wall studs shall be permitted to exceed these limits as permitted by Chapter 6 provisions, provided that story heights are not exceeded. An engineered design shall be provided for the wall or wall framing members where the limits of Chapter 6 are exceeded. Where the story height limits of this section are exceeded, the design of the building, or the noncompliant portions thereof, to resist wind and seismic loads shall be in accordance with the California Building Code.

R301.4 Dead load. The actual weights of materials and construction shall be used for determining dead load with consideration for the dead load of fixed service equipment.

R301.5 Live load. The minimum uniformly distributed live load shall be as provided in Table R301.5.

<table>
<thead>
<tr>
<th>USE</th>
<th>LIVE LOAD</th>
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<tbody>
<tr>
<td>Uninhabitable attics without storage</td>
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<td>Uninhabitable attics with limited storage</td>
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<tr>
<td>Habitable attics and attics served with fixed stairs</td>
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<td>Balconies (exterior) and decks</td>
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<td>Fire escapes</td>
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<td>Guards and handrails</td>
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<tr>
<td>Guard in-fill components</td>
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<tr>
<td>Passenger vehicle garages</td>
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<tr>
<td>Rooms other than sleeping rooms</td>
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<tr>
<td>Sleeping rooms</td>
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<tr>
<td>Stairs</td>
<td>40</td>
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</tbody>
</table>

For SI: 1 pound per square foot = 0.0479 kPa, 1 square inch = 645 mm², 1 pound = 4.45 N.

a. Elevated garage floors shall be capable of supporting a 2000-pound load applied over a 20-square-inch area.

b. Uninhabitable attics without storage are those where the clear height between joists and rafters is not more than 42 inches, or where there are not two or more adjacent trusses with web configurations capable of accommodating an assumed rectangle 42 inches in height by 24 inches in width, or greater, within the plane of the trusses. This live load need not be assumed to act concurrently with any other live load requirements.

c. Individual stair treads shall be designed for the uniformly distributed live load or a 300-pound concentrated load acting over an area of 4 square inches, whichever produces the greater stresses.

d. A single concentrated load applied in any direction at any point along the top.

e. See Section R507.1 for decks attached to exterior walls.

f. Guard in-fill components (all those except the handrail), balusters and panel fillers shall be designed to withstand a horizontally applied normal load of 30 pounds on an area equal to 1 square foot. This load need not be assumed to act concurrently with any other live load requirement.

g. Uninhabitable attics with limited storage are those where the clear height between joists and rafters are 42 inches or greater, or where there are two or more adjacent trusses with web configurations capable of accommodating an assumed rectangle 42 inches in height by 24 inches in width, or greater, within the plane of the trusses.

The live load need only be applied to those portions of the joists or truss bottom chords where all of the following conditions are met:

1. The attic area is accessible from an opening not less than 20 inches in width by 30 inches in length that is located where the clear height in the attic is not less than 30 inches.

2. The slopes of the joists or truss bottom chords are not greater than 2 inches vertical to 12 unis horizontal.

3. Required insulation depth is less than the joist or truss bottom chord member depth.

The remaining portions of the joists or truss bottom chords shall be designed for a uniformly distributed concurrent live load of not less than 10 pounds per square foot.

h. Glazing used in handrail assemblies and guards shall be designed with a safety factor of 4. The safety factor shall be applied to each of the concentrated loads applied to the top of the rail, and to the load on the in-fill components. These loads shall be determined independent of one another, and loads are assumed not to occur with any other live load.
### TABLE R302.6

**DWELLING-GARAGE AND/OR CARPORT SEPARATION**

<table>
<thead>
<tr>
<th>SEPARATION</th>
<th>MATERIAL</th>
</tr>
</thead>
<tbody>
<tr>
<td>From the residence and attics</td>
<td>Not less than (\frac{1}{2})-inch gypsum board or equivalent applied to the garage side</td>
</tr>
<tr>
<td>From habitable rooms above the garage or carport</td>
<td>Not less than (\frac{3}{8})-inch Type X gypsum board or equivalent</td>
</tr>
<tr>
<td>Structure(s) supporting floor/ceiling assemblies used for separation</td>
<td>Not less than (\frac{1}{2})-inch gypsum board or equivalent</td>
</tr>
<tr>
<td>required by this section</td>
<td></td>
</tr>
<tr>
<td>Garages located less than 3 feet from a dwelling unit on the same lot</td>
<td>Not less than (\frac{1}{2})-inch gypsum board or equivalent applied to the interior side of exterior walls that are within this area</td>
</tr>
</tbody>
</table>

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

**R302.7 Under-stair protection.** Enclosed accessible space under stairs shall have walls, under-stair surface and any soffits protected on the enclosed side with \(\frac{1}{2}\)-inch (12.7 mm) gypsum board.

**R302.8 Foam plastics.** For requirements for foam plastics, see Section R316.

**R302.9 Flame spread index and smoke-developed index for wall and ceiling finishes.** Flame spread and smoke developed indexes for wall and ceiling finishes shall be in accordance with Sections R302.9.1 through R302.9.4.

**R302.9.1 Flame spread index.** Wall and ceiling finishes shall have a flame spread index of not greater than 200.

**Exception:** Flame spread index requirements for finishes shall not apply to trim defined as picture molds, chair rails, baseboards and handrails; to doors and windows or their frames; or to materials that are less than \(\frac{1}{2}\)-inch (0.91 mm) in thickness cemented to the surface of walls or ceilings if these materials exhibit flame spread index values not greater than those of paper of this thickness cemented to a noncombustible backing.

**R302.9.2 Smoke-developed index.** Wall and ceiling finishes shall have a smoke-developed index of not greater than 450.

**R302.9.3 Testing.** Tests shall be made in accordance with ASTM E84 or UL 723.

**R302.9.4 Alternative test method.** As an alternative to having a flame spread index of not greater than 200 and a smoke-developed index of not greater than 450 where tested in accordance with ASTM E84 or UL 723, wall and ceiling finishes shall be permitted to be tested in accordance with NFPA 286. Materials tested in accordance with NFPA 286 shall meet the following criteria:

- The interior finish shall comply with the following:
  1. During the 40 kW exposure, flames shall not spread to the ceiling.
  2. The flame shall not spread to the outer extremity of the sample on any wall or ceiling.
  3. Flashover, as defined in NFPA 286, shall not occur.
  4. The peak heat release rate throughout the test shall not exceed 800 kW.

- 5. The total smoke released throughout the test shall not exceed 1,000 m².

**R302.9.5 Stability.** Interior finish materials regulated by this chapter shall be applied or otherwise fastened in such a manner that such materials will not readily become detached where subjected to room temperatures of 200°F (93°C) for not less than 30 minutes.

**R302.10 Flame spread index and smoke-developed index for insulation.** Flame spread and smoke-developed index for insulation shall be in accordance with Sections R302.10.1 through R302.10.5.

**R302.10.1 Insulation.** Insulation materials, including facings, such as vapor retarders and vapor-permeable membranes installed within floor-ceiling assemblies, roof-ceiling assemblies, wall assemblies, crawl spaces and attics shall have a flame spread index not to exceed 25 with an accompanying smoke-developed index not to exceed 450 where tested in accordance with ASTM E84 or UL 723.

**Exceptions:**

1. Where such materials are installed in concealed spaces, the flame spread index and smoke-developed index limitations do not apply to the facings, provided that the facing is installed in substantial contact with the unexposed surface of the ceiling, floor or wall finish.

2. Cellulose fiber loose-fill insulation, that is not spray applied, complying with the requirements of Section R302.10.3, shall not be required to meet a flame spread index requirement but shall be required to meet a smoke-developed index of not more than 450 where tested in accordance with CAN/ULC S102.2.

3. Foam plastic insulation shall comply with Section R316.

**R302.10.2 Loose-fill insulation.** Loose-fill insulation materials that cannot be mounted in the ASTM E84 or UL 723 apparatus without a screen or artificial supports shall comply with the flame spread and smoke-developed limits of Section R302.10.1 where tested in accordance with CAN/ULC S102.2.

**Exception:** Cellulosic fiber loose-fill insulation shall not be required to be tested in accordance with CAN/
ULC S102.2, provided such insulation complies with the requirements of Sections R302.10.1 and R302.10.3.

**R302.10.3 Cellulosic fiber loose-fill insulation.** Cellulosic fiber loose-fill insulation shall comply with CPSC 16 CFR, Parts 1209 and 1404. Each package of such insulating material shall be clearly labeled in accordance with CPSC 16 CFR, Parts 1209 and 1404.

**R302.10.4 Exposed attic insulation.** Exposed insulation materials installed on attic floors shall have a critical radiant flux not less than 0.12 watt per square centimeter.

**R302.10.5 Testing.** Tests for critical radiant flux shall be made in accordance with ASTM E970.

**R302.11 Fireblocking.** In combustible construction, fireblocking shall be provided to cut off both vertical and horizontal concealed draft openings and to form an effective fire barrier between stories, and between a top story and the roof space.

Fireblocking shall be provided in wood-framed construction in the following locations:

1. In concealed spaces of stud walls and partitions, including furred spaces and parallel rows of studs or staggered studs, as follows:
   1.1. Vertically at the ceiling and floor levels.
   1.2. Horizontally at intervals not exceeding 10 feet (3048 mm).

2. At interconnections between concealed vertical and horizontal spaces such as occur at soffits, drop ceilings and cove ceilings.

3. In concealed spaces between stair stringers at the top and bottom of the run. Enclosed spaces under stairs shall comply with Section R302.7.

4. At openings around vents, pipes, ducts, cables and wires at ceiling and floor level, with an approved material to resist the free passage of flame and products of combustion. The material filling this annular space shall not be required to meet the ASTM E136 requirements.

5. For the fireblocking of chimneys and fireplaces, see Section R1003.19.

6. Fireblocking of cornices of a two-family dwelling is required at the line of dwelling unit separation.

**R302.11.1 Fireblocking materials.** Except as provided in Section R302.11, Item 4, fireblocking shall consist of the following materials.

1. Two-inch (51 mm) nominal lumber.

2. Two thicknesses of 1-inch (25.4 mm) nominal lumber with broken lap joints.

3. One thickness of 1/2-inch (18.3 mm) wood structural panels with joints backed by 1/2-inch (18.3 mm) wood structural panels.

4. One thickness of 1/4-inch (19.1 mm) particleboard with joints backed by 1/4-inch (19.1 mm) particleboard.

5. One-half-inch (12.7 mm) gypsum board.

6. One-quarter-inch (6.4 mm) cement-based millboard.

7. Batt or blankets of mineral wool or glass fiber or other approved materials installed in such a manner as to be securely retained in place.

8. Cellulose insulation installed as tested in accordance with ASTM E119 or UL 263, for the specific application.

**R302.11.1.1 Batt or blankets of mineral or glass fiber.** Batt or blankets of mineral or glass fiber or other approved nonrigid materials shall be permitted for compliance with the 10-foot (3048 mm) horizontal fireblocking in walls constructed using parallel rows of studs or staggered studs.

**R302.11.1.2 Unfaced fiberglass.** Unfaced fiberglass batt insulation used as fireblocking shall fill the entire cross section of the wall cavity to a height of not less than 16 inches (406 mm) measured vertically. Where piping, conduit or similar obstructions are encountered, the insulation shall be packed tightly around the obstruction.

**R302.11.1.3 Loose-fill insulation material.** Loose-fill insulation material shall not be used as a fireblock unless specifically tested in the form and manner intended for use to demonstrate its ability to remain in place and to retard the spread of fire and hot gases.

**R302.11.2 Fireblocking integrity.** The integrity of fireblocks shall be maintained.

**R302.12 Draftstopping.** In combustible construction where there is usable space both above and below the concealed space of a floor-ceiling assembly, draftstops shall be installed so that the area of the concealed space does not exceed 1,000 square feet (92.9 m²). Draftstopping shall divide the concealed space into approximately equal areas. Where the assembly is enclosed by a floor membrane above and a ceiling membrane below, draftstopping shall be provided in floor-ceiling assemblies under the following circumstances:

1. Ceiling is suspended under the floor framing.

2. Floor framing is constructed of truss-type open-web or perforated members.

**R302.12.1 Materials.** Draftstopping materials shall be not less than 1/2-inch (12.7 mm) gypsum board, 1/4-inch (9.5 mm) wood structural panels or other approved materials adequately supported. Draftstopping shall be installed parallel to the floor framing members unless otherwise approved by the building official. The integrity of the draftstops shall be maintained.

**R302.13 Fire protection of floors.** Floor assemblies that are not required elsewhere in this code to be fire-resistance rated, shall be provided with a 1/2-inch (12.7 mm) gypsum wallboard membrane, 1/4-inch (16 mm) wood structural panel membrane, or equivalent on the underside of the floor framing member. Penetrations or openings for ducts, vents, electrical outlets, lighting, devices, luminaires, wires, speakers,
Section 17958.1, efficiency dwelling units shall comply with the following:

1. The unit shall have a living room of not less than 220 square feet (20.4 m²) of floor area. An additional 100 square feet (9.3 m²) of floor area shall be provided for each occupant of such unit in excess of two.

2. The unit shall be provided with a separate closet.

3. The unit shall be provided with a kitchen sink, cooking appliance and refrigeration facilities, each having a clear working space of not less than 30 inches (762 mm) in front. Light and ventilation conforming to this code shall be provided.

4. The unit shall be provided with a separate bathroom containing a water closet, lavatory and bathtub or shower.

SECTION R305
CEILING HEIGHT

R305.1 Minimum height. Habitable space, hallways and portions of basements containing these spaces shall have a ceiling height of not less than 7 feet (2134 mm). Bathrooms, toilet rooms and laundry rooms shall have a ceiling height of not less than 6 feet 8 inches (2032 mm).

Exceptions:

1. For rooms with sloped ceilings, the required floor area of the room shall have a ceiling height of not less than 5 feet (1524 mm) and not less than 50 percent of the required floor area shall have a ceiling height of not less than 7 feet (2134 mm).

2. The ceiling height above bathroom and toilet room fixtures shall be such that the fixture is capable of being used for its intended purpose. A shower or tub equipped with a showerhead shall have a ceiling height of not less than 6 feet 8 inches (2032 mm) above an area of not less than 30 inches (762 mm) by 30 inches (762 mm) at the showerhead.

3. Beams, girders, ducts or other obstructions in basements containing habitable space shall be permitted to project to within 6 feet 4 inches (1931 mm) of the finished floor.

R305.1.1 Basements. Portions of basements that do not contain habitable space or hallways shall have a ceiling height of not less than 6 feet 8 inches (2032 mm).

Exception: At beams, girders, ducts or other obstructions, the ceiling height shall be not less than 6 feet 4 inches (1931 mm) from the finished floor.

SECTION R306
SANITATION

R306.1 Toilet facilities. Every dwelling unit shall be provided with a water closet, lavatory, and a bathtub or shower.

R306.2 Kitchen. Each dwelling unit shall be provided with a kitchen area and every kitchen area shall be provided with a sink.

R306.3 Sewage disposal. Plumbing fixtures shall be connected to a sanitary sewer or to an approved private sewage disposal system.

R306.4 Water supply to fixtures. Plumbing fixtures shall be connected to an approved water supply. Kitchen sinks, lavatories, bathtubs, showers, bidets, laundry tubs and washing machine outlets shall be provided with hot and cold water.

SECTION R307
TOILET, BATH AND SHOWER SPACES

R307.1 Space required. Fixtures shall be spaced in accordance with the California Plumbing Code.

R307.2 Bathtub and shower spaces. Bathtub and shower floors and walls above bathtubs with installed shower heads and in shower compartments shall be finished with a nonabsorbent surface. Such wall surfaces shall extend to a height of not less than 6 feet (1829 mm) above the floor.

SECTION R308
GLAZING

R308.1 Identification. Except as indicated in Section R308.1.1 each pane of glazing installed in hazardous locations as defined in Section R308.4 shall be provided with a manufacturer's designation specifying who applied the designation, designating the type of glass and the safety glazing standard with which it complies, which is visible in the final installation. The designation shall be acid etched, sandblasted, ceramic-fired, laser etched, embossed, or be of a type that once applied cannot be removed without being destroyed. A label shall be permitted in lieu of the manufacturer's designation.

Exceptions:

1. For other than tempered glass, manufacturer's designations are not required provided that the building official approves the use of a certificate, affidavit or other evidence confirming compliance with this code.

2. Tempered spandrel glass is permitted to be identified by the manufacturer with a removable paper designation.

R308.1.1 Identification of multiple assemblies. Multipane assemblies having individual panes not exceeding 1 square foot (0.09 m²) in exposed area shall have not less than one pane in the assembly identified in accordance with Section R308.1. Other panes in the assembly shall be labeled "CPSC 16 CFR 1201" or "ANSI Z97.1" as appropriate.

R308.2 Louvered windows or jalousies. Regular, float, wired or patterned glass in jalousies and louvered windows shall be not less than nominal 1/16 inch (5 mm) thick and not more than 48 inches (1219 mm) in length. Exposed glass edges shall be smooth.
R308.2.1 Wired glass prohibited. Wired glass with wire exposed on longitudinal edges shall not be used in jalousies or louvered windows.

R308.3 Human impact loads. Individual glazed areas, including glass mirrors in hazardous locations such as those indicated as defined in Section R308.4, shall pass the test requirements of Section R308.3.1.

Exceptions:
1. Louvered windows and jalousies shall comply with Section R308.2.
2. Mirrors and other glass panels mounted or hung on a surface that provides a continuous backing support.
3. Glass unit masonry complying with Section R607.

R308.3.1 Impact test. Where required by other sections of the code, glazing shall be tested in accordance with CPSC 16 CFR 1201. Glazing shall comply with the test criteria for Category II unless otherwise indicated in Table R308.3.1(1).

Exception: Glazing not in doors or enclosures for hot tubs, whirlpools, saunas, steam rooms, bathtubs and showers shall be permitted to be tested in accordance with ANSI Z97.1. Glazing shall comply with the test criteria for Class A unless indicated in Table R308.3.1(2).

R308.4 Hazardous locations. The locations specified in Sections R308.4.1 through R308.4.7 shall be considered to be specific hazardous locations for the purposes of glazing.

R308.4.1 Glazing in doors. Glazing in fixed and operable panels of swinging, sliding and bifold doors shall be considered to be a hazardous location.

Exceptions:
1. Glazed openings of a size through which a 3-inch-diameter (76 mm) sphere is unable to pass.
2. Decorative glazing.

R308.4.2 Glazing adjacent to doors. Glazing in an individual fixed or operable panel adjacent to a door shall be considered to be a hazardous location where the bottom exposed edge of the glazing is less than 60 inches (1524 mm) above the floor or walking surface and it meets either of the following conditions:

1. Where the glazing is within 24 inches (610 mm) of either side of the door in the plane of the door in a closed position.
2. Where the glazing is on a wall perpendicular to the plane of the door in a closed position and within 24 inches (610 mm) of the hinge side of an in-swinging door.

Exceptions:
1. Decorative glazing.
2. Where there is an intervening wall or other permanent barrier between the door and the glazing.
3. Where access through the door is to a closet or storage area 3 feet (914 mm) or less in depth. Glazing in this application shall comply with Section R308.4.3.
4. Glazing that is adjacent to the fixed panel of patio doors.

R308.4.3 Glazing in windows. Glazing in an individual fixed or operable panel that meets all of the following conditions shall be considered to be a hazardous location:

1. The exposed area of an individual pane is larger than 9 square feet (0.836 m²),
2. The bottom edge of the glazing is less than 18 inches (457 mm) above the floor,
3. The top edge of the glazing is more than 36 inches (914 mm) above the floor; and

<table>
<thead>
<tr>
<th>EXPOSED SURFACE AREA OF ONE SIDE OF ONE LITE</th>
<th>GLAZING IN STORM OR COMBINATION DOORS (Category Class)</th>
<th>GLAZING IN DOORS (Category Class)</th>
<th>GLAZED PANELS REGULATED BY SECTION R308.4.3 (Category Class)</th>
<th>GLAZED PANELS REGULATED BY SECTION R308.4.2 (Category Class)</th>
<th>GLAZING IN DOORS AND ENCLOSURES REGULATED BY SECTION R308.4.5 (Category Class)</th>
<th>SLIDING GLASS DOORS PATIO TYPE (Category Class)</th>
</tr>
</thead>
<tbody>
<tr>
<td>9 square feet or less</td>
<td>I</td>
<td>I</td>
<td>NR</td>
<td>I</td>
<td>II</td>
<td>II</td>
</tr>
<tr>
<td>More than 9 square feet</td>
<td>I</td>
<td>I</td>
<td>II</td>
<td>II</td>
<td>II</td>
<td>II</td>
</tr>
</tbody>
</table>

For SI: 1 square foot = 0.0929 m².
NR = "No Requirement."

<table>
<thead>
<tr>
<th>EXPOSED SURFACE AREA OF ONE SIDE OF ONE LITE</th>
<th>GLAZED PANELS REGULATED BY SECTION R308.4.3 (Category Class)</th>
<th>GLAZED PANELS REGULATED BY SECTION R308.4.2 (Category Class)</th>
<th>DOORS AND ENCLOSURES REGULATED BY SECTION R308.4.5 (Category Class)</th>
</tr>
</thead>
<tbody>
<tr>
<td>9 square feet or less</td>
<td>No requirement</td>
<td>B</td>
<td>A</td>
</tr>
<tr>
<td>More than 9 square feet</td>
<td>A</td>
<td>A</td>
<td>A</td>
</tr>
</tbody>
</table>

For SI: 1 square foot = 0.0929 m².
a. Use is permitted only by the exception to Section R308.3.1.
R309.4 Automatic garage door openers. Automatic garage door openers, if provided, shall be listed and labeled in accordance with UL 325. See Health and Safety Code Sections 19890 and 19891 for additional provisions for residential garage door openers.

R309.5 Fire sprinklers location on property. Private garages shall be protected by fire sprinklers where the garage wall has been designed based on Table R302.1(2), Footnote a. Sprinklers in garages shall be connected to an automatic sprinkler system that complies with Section R313. Garage sprinklers shall be residential sprinklers or quick-response sprinklers, designed to provide a density of 0.05 gpm/ft². Garage doors shall not be considered obstructions with respect to sprinkler placement.

R309.6 Fire sprinklers, attached garages, and carpports with habitable space above. Attached garages and carpports with habitable space above shall be protected by fire sprinklers in accordance with this section and Section R313. Protection shall be provided in accordance with one of the following:

1. Residential sprinklers installed in accordance with their listing.

2. Extended coverage sprinklers discharging water not less than their listed flow rate for Light Hazard in accordance with NFPA 13.

3. Quick-response spray sprinklers at light hazard spacing in accordance with NFPA 13 designed to discharge at 0.05 gpm/ft² density (minimum).

The system demand shall be permitted to be limited to the number of sprinklers in the compartment but shall not exceed two sprinklers for hydraulic calculation purposes. Garage doors shall not be considered obstructions and shall be permitted to be ignored for placement and calculation of sprinklers.

Exception: An automatic residential fire sprinkler system shall not be required when additions or alterations are made to existing carpports and/or garages that do not have an automatic residential fire sprinkler system installed in accordance with this section.

R309.7 Extension garage door springs. Every extension garage door spring sold or offered for sale, whether new or sold as a replacement, or installed in any garage or carpport which is accessory to a dwelling covered by this code, shall conform to the requirements for garage door springs located in Section 1211 of the California Building Code.

R309.8 Electric vehicle (EV) charging infrastructure. Newly constructed one- and two-family dwellings and townhouses with attached private garages shall comply with EV infrastructure requirements in accordance with the California Green Building Standards Code, Chapter 4, Division 4.1.

SECTION R310
EMERGENCY ESCAPE AND RESCUE OPENINGS

R310.1 Emergency escape and rescue opening required. Basements, habitable attics and every sleeping room shall have not less than one operable emergency escape and rescue opening. Where basements contain one or more sleeping rooms, an emergency escape and rescue opening shall be required in each sleeping room. Emergency escape and rescue openings shall open directly into a public way, or to a yard or court that opens to a public way.

Exception: Storm shelters and basements used only to house mechanical equipment not exceeding a total floor area of 200 square feet (18.58 m²).

R310.1.1 Operational constraints and opening control devices. Emergency escape and rescue openings shall be maintained free of any obstructions other than those allowed by this section and shall be operational from the inside of the room without the use of keys, tools or special knowledge. Window opening control devices complying with ASTM F2090 shall be permitted for use on windows serving as a required emergency escape and rescue opening.

R310.2 Emergency escape and rescue openings. Emergency escape and rescue openings shall have minimum dimensions as specified in this section.

R310.2.1 Minimum opening area. Emergency and escape rescue openings shall have a net clear opening of not less than 5.7 square feet (0.530 m²). The net clear opening dimensions required by this section shall be obtained by the normal operation of the emergency escape and rescue opening from the inside. The net clear height opening shall be not less than 24 inches (610 mm) and the net clear width shall be not less than 20 inches (508 mm).

Exception: Grade floor or below grade openings shall have a net clear opening of not less than 5 square feet (0.465 m²).

R310.2.2 Window sill height. Where a window is provided as the emergency escape and rescue opening, it shall have the bottom of the clear opening not greater than 44 inches (1118 mm) measured from the floor; where the sill height is below grade, it shall be provided with a window well in accordance with Section R310.2.3.

R310.2.3 Window wells. The horizontal area of the window well shall be not less than 9 square feet (0.9 m²), with a horizontal projection and width of not less than 36 inches (914 mm). The area of the window well shall allow the emergency escape and rescue opening to be fully opened.

Exception: The ladder or steps required by Section R310.2.3.1 shall be permitted to encroach not more than 6 inches (152 mm) into the required dimensions of the window well.

R310.2.3.1 Ladder and steps. Window wells with a vertical depth greater than 44 inches (1118 mm) shall be equipped with a permanently affixed ladder or steps usable with the window in the fully open position. Ladders or steps required by this section shall not be required to comply with Sections R311.7 and R311.8. Ladders or rungs shall have an inside width of not less than 12 inches (305 mm), shall project not less than 3 inches (76 mm) from the wall and shall be spaced not more than 18 inches (457 mm) on center vertically for the full height of the window well.

R310.2.3.2 Drainage. Window wells shall be designed for proper drainage by connecting to the building's foundation drainage system required by Section R405.1 or by an approved alternative method.

Exception: A drainage system for window wells is not required where the foundation is on well-drained soil or sand-gravel mixture soils in accordance with
the United Soil Classification System, Group I Soils, as detailed in Table R405.1.

R310.2.5 Replacement windows. Replacement windows installed in buildings meeting the scope of this code shall be exempt from the maximum sill height requirements of Sections R310.1 and Sections R310.2.1 and R310.2.2, provided the replacement window meets the following conditions:

1. The replacement window is the manufacturer’s largest standard size window that will fit within the existing frame or existing rough opening. The replacement window is of the same operating style as the existing window or a style that provides for an equal or greater window opening area than the existing window.

2. The replacement window is not part of a change of occupancy.

R310.3 Emergency escape and rescue doors. Where a door is provided as the required emergency escape and rescue opening, it shall be permitted to be a side-hinged door or a slider. Where the opening is below the adjacent ground elevation, it shall be provided with a bulkhead enclosure.

R310.3.1 Minimum door opening size. The minimum net clear height opening for any door that serves as an emergency and escape rescue opening shall be in accordance with Section R310.2.1.

R310.3.2 Bulkhead enclosures. Bulkhead enclosures shall provide direct access from the basement. The bulkhead enclosure shall provide the minimum net clear opening equal to the door in the fully open position.

R310.3.2.1 Drainage. Bulkhead enclosures shall be designed for proper drainage by connecting to the building’s foundation drainage system required by Section R405.1 or by an approved alternative method.

Exception: A drainage system for bulkhead enclosures is not required where the foundation is on well-drained soil or sand-gravel mixture soils in accordance with the United Soil Classification System, Group I Soils, as detailed in Table R405.1.

R310.4 Bars, grilles, covers and screens. Bars, grilles, covers, screens or similar devices are permitted to be placed over emergency escape and rescue openings, bulkhead enclosures, or window wells that serve such openings, provided that the minimum net clear opening size complies with Sections R310.1.1 to R310.2.3, and such devices shall be releasable or removable from the inside without the use of a key, tool, special knowledge or force greater than that required for the normal operation of the escape and rescue opening. The release mechanism shall be maintained operable at all times.

Such bars, grills, grates or any similar devices shall be equipped with an approved exterior release device for use by the fire department only when required by the authority having jurisdiction.

Where security bars (burglar bars) are installed on emergency egress and rescue windows or doors, or after July 1, 2000, such devices shall comply with California Building Standards Code, Part 12, Chapter 12-3 and other applicable provisions of this code.

R310.5 Dwelling additions. Where dwelling additions occur that contain sleeping rooms, an emergency escape and rescue opening shall be provided in each new sleeping room. Where dwelling additions occur that have basements, an emergency escape and rescue opening shall be provided in the new basement.

Exceptions:

1. An emergency escape and rescue opening is not required in a new basement that contains a sleeping room with an emergency escape and rescue opening.

2. An emergency escape and rescue opening is not required in a new basement where there is an emergency escape and rescue opening in an existing basement that is accessible from the new basement.

R310.6 Alterations or repairs of existing basements. An emergency escape and rescue opening is not required where existing basements undergo alterations or repairs.

Exception: New sleeping rooms created in an existing basement shall be provided with emergency escape and rescue openings in accordance with Section R310.1.

SECTION R311
MEANS OF EGRESS

R311.1 Means of egress. Dwellings shall be provided with a means of egress in accordance with this section. The means of egress shall provide a continuous and unobstructed path of vertical and horizontal egress travel from all portions of the dwelling to the required egress door without requiring travel through a garage. The required egress door shall open directly into a public way or to a yard or court that opens to a public way.

R311.2 Egress door. Not less than one egress door shall be provided for each dwelling unit. The egress door shall be side-hinged, shall provide a clear width of not less than 32 inches (813 mm) where measured between the face of the door and the stop, with the door open 90 degrees (1.57 rad). The clear height of the door opening shall be not less than 78 inches (1981 mm) in height measured from the top of the threshold to the bottom of the stop. Other doors shall not be required to comply with these minimum dimensions. Egress doors shall be readily openable from inside the dwelling without the use of a key or special knowledge or effort.

R311.3 Floors and landings at exterior doors. There shall be a landing or floor on each side of each exterior door. The width of each landing shall be not less than the door served. Every landing shall have a dimension of not less than 36 inches (914 mm) measured in the direction of travel. The slope at exterior landings shall not exceed 1/4 unit vertical in 12 units horizontal (2 percent).

Exception: Exterior balconies less than 60 square feet (5.6 m²) and only accessible from a door are permitted to have a landing less than 36 inches (914 mm) measured in the direction of travel.

R311.3.1 Floor elevations at the required egress doors. Landings or finished floors at the required egress door shall be not more than 1 1/2 inches (38 mm) lower than the top of the threshold.

Exception: The landing or floor on the exterior side shall be not more than 7/16 inches (196 mm) below the
is useable solely for vehicle parking, building access or limited storage provided that such enclosure is not built so as to render the building or structure in violation of this section.

**R322.1.6 Protection of mechanical, plumbing and electrical systems.** Electrical systems, equipment and components; heating, ventilating, air conditioning; plumbing appliances and plumbing fixtures; duct systems; and other service equipment shall be located at or above the elevation required in Section R322.2 or R322.3. If replaced as part of a substantial improvement, electrical systems, equipment and components; heating, ventilating, air conditioning and plumbing appliances and plumbing fixtures; duct systems; and other service equipment shall meet the requirements of this section. Systems, fixtures, and equipment and components shall not be mounted on or penetrate through walls intended to break away under flood loads.

**Exception:** Locating electrical systems, equipment and components; heating, ventilating, air conditioning; plumbing appliances and plumbing fixtures; duct systems; and other service equipment is permitted below the elevation required in Section R322.2 or R322.3 provided that they are designed and installed to prevent water from entering or accumulating within the components and to resist hydrostatic and hydrodynamic loads and stresses, including the effects of buoyancy, during the occurrence of flooding to the design flood elevation in accordance with ASCE 24. Electrical wiring systems are permitted to be located below the required elevation provided that they conform to the provisions of the California Electrical Code for wet locations.

**R322.1.7 Protection of water supply and sanitary sewage systems.** Water supply systems shall be designed to minimize or eliminate infiltration of flood waters into the water supply and distribution system. Sanitary sewage systems shall be designed to minimize or eliminate infiltration of floodwaters into sanitary drainage systems and discharges from sanitary drainage systems into floodwaters.

**R322.1.8 Flood-resistant materials.** Building materials and installation methods used for flooring and interior and exterior walls and wall coverings below the elevation required in Section R322.2 or R322.3 shall be flood damage-resistant materials that conform to the provisions of FEMA TB-2.

**R322.1.9 Manufactured homes.** *(Not adopted in CA)*

**R322.1.10 As-built elevation documentation.** A registered design professional shall prepare and seal documentation of the elevations specified in Section R322.2 or R322.3.

**R322.2 Flood hazard areas (including A Zones).** Areas that have been determined to be prone to flooding and that are not subject to high-velocity wave action shall be designated as flood hazard areas. Flood hazard areas that have been delineated as subject to wave heights between 1/125 feet (457 mm) and 3 feet (914 mm) or otherwise designated by the jurisdiction shall be designated as Coastal A Zones and are subject to the requirements of Section R322.3. Buildings and structures constructed in whole or in part in flood hazard areas shall be designed and constructed in accordance with Sections R322.2.1 through R322.2.3.

**R322.2.1 Elevation requirements.**

1. Buildings and structures in flood hazard areas, including flood hazard areas designated as Coastal A Zones, shall have the lowest floors elevated to or above the base flood elevation plus 1 foot (305 mm), or the design flood elevation, whichever is higher.

2. In areas of shallow flooding (AO Zones), buildings and structures shall have the lowest floor (including basement) elevated to a height above the highest adjacent grade of not less than the depth number specified in feet (mm) on the FIRM plus 1 foot (305 mm), or not less than 3 feet (915 mm) if a depth number is not specified.

3. Basement floors that are below grade on all sides shall be elevated to or above base flood elevation plus 1 foot (305 mm), or the design flood elevation, whichever is higher.

**Exception:** Enclosed areas below the design flood elevation, including basements with floors that are not below grade on all sides, shall meet the requirements of Section R322.2.2.

**R322.2.2 Enclosed area below design flood elevation.** Enclosed areas, including crawl spaces, that are below the design flood elevation shall:

1. Be used solely for parking of vehicles, building access or storage.

2. Be provided with flood openings that meet the following criteria and are installed in accordance with Section R322.2.2.1:

   2.1. The total net area of non-engineered openings shall be not less than 1 square inch (645 mm²) for each square foot (0.093 m²) of enclosed area where the enclosed area is measured on the exterior of the enclosure walls, or the openings shall be designed as engineered openings and the construction documents shall include a statement by a registered design professional that the design of the openings will provide for equalization of hydrostatic flood forces on exterior walls by allowing for the automatic entry and exit of floodwaters as specified in Section 2.7.2.2 of ASCE 24.

   2.2. Openings shall be not less than 3 inches (76 mm) in any direction in the plane of the wall.

   2.3. The presence of louvers, blades, screens and faceplates or other covers and devices shall allow the automatic flow of floodwater into and out of the enclosed areas and shall be accounted for in the determination of the net open area.

**R322.2.2.1 Installation of openings.** The walls of enclosed areas shall have openings installed such that:

1. There shall be not less than two openings on different sides of each enclosed area; if a building...
has more than one enclosed area below the design flood elevation, each area shall have openings.

2. The bottom of each opening shall be not more than 1 foot (305 mm) above the higher of the final interior grade or floor and the finished exterior grade immediately under each opening.

3. Openings shall be permitted to be installed in doors and windows; doors and windows without installed openings do not meet the requirements of this section.

R322.2.3 Foundation design and construction. Foundation walls for buildings and structures erected in flood hazard areas shall meet the requirements of Chapter 4.

Exception: Unless designed in accordance with Section R404:

1. The unsupported height of 6-inch (152 mm) plain masonry walls shall be not more than 3 feet (914 mm).

2. The unsupported height of 8-inch (203 mm) plain masonry walls shall be not more than 4 feet (1219 mm).

3. The unsupported height of 8-inch (203 mm) reinforced masonry walls shall be not more than 8 feet (2438 mm).

For the purpose of this exception, unsupported height is the distance from the finished grade of the under-floor space to the top of the wall.

R322.2.4 Tanks. Underground tanks shall be anchored to prevent flotation, collapse and lateral movement under conditions of the base flood. Above-ground tanks shall be installed at or above the elevation required in Section R322.2.1 or shall be anchored to prevent flotation, collapse and lateral movement under conditions of the base flood.

R322.3 Coastal high-hazard areas (including V Zones and Coastal A Zones, where designated). Areas that have been determined to be subject to wave heights in excess of 3 feet (914 mm) or subject to high-velocity wave action or wave-induced erosion shall be designated as coastal high-hazard areas. Flood hazard areas that have been designated as subject to wave heights between 11/2 feet (457 mm) and 3 feet (914 mm) or otherwise designated by the jurisdiction shall be designated as Coastal A Zones. Buildings and structures constructed in whole or in part in coastal high-hazard areas and coastal A Zones, where designated, shall be designed and constructed in accordance with Sections R322.3.1 through R322.3.7.

R322.3.1 Location and site preparation.

1. New buildings and buildings that are determined to be substantially improved pursuant to Section R105.3.1.1 shall be located landward of the reach of mean high tide.

2. For any alteration of sand dunes and mangrove stands, the building official shall require submission of an engineering analysis that demonstrates that the proposed alteration will not increase the potential for flood damage.

R322.3.2 Elevation requirements.

1. Buildings and structures erected within coastal high-hazard areas and Coastal A Zones, shall be elevated so that the bottom of the lowest horizontal structural members supporting the lowest floor, with the exception of piling, pile caps, columns, grade beams and bracing, is elevated to or above the base flood elevation plus 1 foot (305 mm) or the design flood elevation, whichever is higher.

2. Basement floors that are below grade on all sides are prohibited.

3. The use of fill for structural support is prohibited.

4. Minor grading, and the placement of minor quantities of fill, shall be permitted for landscaping and for drainage purposes under and around buildings and for support of parking slabs, pool decks, patios and walkways.

5. Walls and partitions enclosing areas below the design flood elevation shall meet the requirements of Sections R322.3.4 and R322.3.5.

R322.3.3 Foundations. Buildings and structures erected in coastal high-hazard areas and Coastal A Zones shall be supported on pilings or columns and shall be adequately anchored to such pilings or columns. The space below the elevated building shall be either free of obstruction or, if enclosed with walls, the walls shall meet the requirements of Section R322.3.4. Pilings shall have adequate soil penetrations to resist the combined wave and wind loads (lateral and uplift). Water-loading values used shall be those associated with the design flood. Wind-loading values shall be those required by this code. Pile embedment shall include consideration of decreased resistance capacity caused by scour of soil strata surrounding the piling. Pile systems design and installation shall be certified in accordance with Section R322.3.6. Spread footing, mat, raft or other foundations that support columns shall not be permitted where soil investigations that are required in accordance with Section R401.4 indicate that soil material under the spread footing, mat, raft or other foundation is subject to scour or erosion from wave-velocity flow conditions. If permitted, spread footing, mat, raft or other foundations that support columns shall be designed in accordance with ASCE 24. Slabs, pools, pool decks and walkways shall be located and constructed to be structurally independent of buildings and structures and their foundations to prevent transfer of flood loads to the buildings and structures during conditions of flooding, scour or erosion from wave-velocity flow conditions, unless the buildings and structures and their foundations are designed to resist the additional load.

Exception: In Coastal A Zones, stem wall foundations supporting a floor system above and backfilled with soil or gravel to the underside of the floor system shall be permitted provided the foundations are designed to account for wave action, debris impact, erosion and
local scour. Where soils are susceptible to erosion and local scour, stem wall foundations shall have deep footings to account for the loss of soil.

R322.3.4 Walls below design flood elevation. Walls and partitions are permitted below the elevated floor, provided that such walls and partitions are not part of the structural support of the building or structure and:

1. Electrical, mechanical and plumbing system components are not to be mounted on or penetrate through walls that are designed to break away under flood loads; and

2. Are constructed with insect screening or open lattice; or

3. Are designed to break away or collapse without causing collapse, displacement or other structural damage to the elevated portion of the building or supporting foundation system. Such walls, framing and connections shall have a resistance of not less than 10 (479 Pa) and not more than 20 pounds per square foot (958 Pa) as determined using allowable stress design; or

4. Where wind loading values of this code exceed 20 pounds per square foot (958 Pa), as determined using allowable stress design, the construction documents shall include documentation prepared and sealed by a registered design professional that:

   4.1. The walls and partitions below the design flood elevation have been designed to collapse from a water load less than that which would occur during the base flood.

   4.2. The elevated portion of the building and supporting foundation system have been designed to withstand the effects of wind and flood loads acting simultaneously on structural and nonstructural building components. Water-loading values used shall be those associated with the design flood. Wind-loading values shall be those required by this code.

5. Walls intended to break away under flood loads as specified in Item 3 or 4 have flood openings that meet the criteria in Section R322.2.2, Item 2.

R322.3.5 Enclosed areas below design flood elevation. Enclosed areas below the design flood elevation shall be used solely for parking of vehicles, building access or storage.

R322.3.5.1 Protection of building envelope. An exterior door that meets the requirements of Section R609 shall be installed at the top of stairs that provide access to the building and that are enclosed with walls designed to break away in accordance with Section R322.3.4.

R322.3.6 Construction documents. The construction documents shall include documentation that is prepared and sealed by a registered design professional that the design and methods of construction to be used meet the applicable criteria of this section.

R322.3.7 Tanks. Underground tanks shall be anchored to prevent flotation, collapse and lateral movement under conditions of the base flood. Above-ground tanks shall be installed at or above the elevation required in Section R322.3.2. Where elevated on platforms, the platforms shall be cantilevered from or knee braced to the building or shall be supported on foundations that conform to the requirements of Section R322.3.

SECTION R323
STORM SHELTERS

R323.1 General. This section applies to storm shelters where constructed as separate detached buildings or where constructed as safe rooms within buildings for the purpose of providing refuge from storms that produce high winds, such as tornados and hurricanes. In addition to other applicable requirements in this code, storm shelters shall be constructed in accordance with ICC/NSSA-500.

SECTION 324
SOLAR ENERGY SYSTEMS

R324.1 General. Solar energy systems shall comply with the provisions of this section.

R324.2 Solar thermal systems. Solar thermal systems shall be designed and installed in accordance with the California Plumbing Code and the California Fire Code.

R324.3 Photovoltaic systems. Photovoltaic systems shall be designed and installed in accordance with Sections R324.3.1 through R324.6.1 and the California Electrical Code. Inverters shall be listed and labeled in accordance with UL 1741. Systems connected to the utility grid shall use inverters listed for utility interaction.

R324.3.1 Equipment listings. Photovoltaic panels and modules shall be listed and labeled in accordance with UL 1703.

R324.4 Rooftop-mounted photovoltaic systems. Rooftop-mounted photovoltaic panel systems installed on or above the roof covering shall be designed and installed in accordance with Section R907.

R324.4.1 Roof live load. Roof structures that provide support for photovoltaic panel systems shall be designed for applicable roof live load. The design of roof structures need not include roof live load in the areas covered by photovoltaic panel systems. Portions of roof structures not covered by photovoltaic panels shall be designed for roof live load. Roof structures that provide support for photovoltaic panel systems shall be designed for live load, Lg, for the load case where the photovoltaic panel system is not present.

R324.5 Building-integrated photovoltaic systems. Building-integrated photovoltaic systems that serve as roof coverings shall be designed and installed in accordance with Section R905.

R324.5.1 Photovoltaic shingles. Photovoltaic shingles shall comply with Section R905.16.
R324.6 Ground-mounted photovoltaic systems. Ground-mounted photovoltaic systems shall be designed and installed in accordance with Section R301.

R324.6.1 Fire separation distances. Ground-mounted photovoltaic systems shall be subject to the fire separation distance requirements determined by the enforcing agency.

R324.7 Access and pathways. Roof access, pathways and spacing requirements shall be provided in accordance with Sections R324.7.1 through R324.7.2.7.

Exceptions:

1. Detached, nonhabitable Group U Structures including, but not limited to, garages and accessory structures to one- and two-family dwellings and townhouses, such as parking shade structures, carports, solar trellises and similar structures.

2. Roof access, pathways and spacing requirements need not be provided where an alternative ventilation method approved by the enforcing agency has been provided or where the enforcing agency has determined that vertical ventilation techniques will not be employed.

R324.7.1 Roof access points. Roof access points shall be located in areas that do not require the placement of ground ladders over openings such as windows or doors, and located at strong points of building construction in locations where the access point does not conflict with overhead obstructions such as tree limbs, wires or signs.

R324.7.2 Solar photovoltaic systems. Solar photovoltaic systems shall comply with Sections R324.7.2.1 through R324.7.2.7.

R324.7.2.1 Size of solar photovoltaic array. Each photovoltaic array shall be limited to 150 feet by 150 feet (45 720 by 45 720 mm). Multiple arrays shall be separated by a clear access pathway not less than 3 feet (914 mm) in width.

R324.7.2.2 Hip roof layouts. Panels and modules installed on dwellings with hip roof layouts shall be located in a manner that provides a clear access pathway not less than 3 feet (914 mm) in width from the eave to the ridge on each roof slope where panels and modules are located. The access pathway shall be located at a structurally strong location on the building capable of supporting the live load of fire fighters accessing the roof.

Exception: These requirements shall not apply to roofs with slopes of 2 units vertical in 12 units horizontal (16.6 percent) and less.

R324.7.2.3 Single ridge roofs. Panels and modules installed on dwellings with a single ridge shall be located in a manner that provides two, 3-foot-wide (914 mm) access pathways from the eave to the ridge on each roof slope where panels or modules are located.

Exception: This requirement shall not apply to roofs with slopes of 2 units vertical in 12 units horizontal (16.6 percent) and less.

R324.7.2.4 Roofs with hips and valleys. Panels and modules installed on dwellings with roof hips or valleys shall not be located less than 18 inches (457 mm) from a hip or valley where panels or modules are to be placed on both sides of a hip or valley. Where panels are to be located on one side only of a hip or valley that is of equal length, the 18-inch (457 mm) clearance does not apply.

Exception: These requirements shall not apply to roofs with slopes of 2 units vertical in 12 units horizontal (16.6 percent) and less.

R324.7.2.5 Allowance for smoke ventilation operations. Panels and modules installed on dwellings shall not be located less than 3 feet (914 mm) below the roof ridge to allow for fire department smoke ventilation operations.

Exception: Where an alternative ventilation method approved by the enforcing agency has been provided or where the enforcing agency has determined that vertical ventilation techniques will not be employed, clearance from the roof ridge is not required.

R324.7.2.6 Ground-mounted photovoltaic arrays. Ground mounted photovoltaic arrays shall comply with this section and the California Electrical Code. Setback requirements shall not apply to ground-mounted, free-standing photovoltaic arrays. A clear, brush-free area of 10 feet (3048 mm) shall be required for ground-mounted photovoltaic arrays.

R324.7.2.7 Locations of DC conductors. Conduit, wiring systems, and raceways for photovoltaic circuits shall be located as close as possible to the ridge or hip or valley and from the hip or valley as directly as possible to an outside wall to reduce trip hazards and maximize ventilation opportunities. Conduit runs between sub arrays and to DC combiner boxes shall be installed in a manner that minimizes the total amount of conduit on the roof by taking the shortest path from the array to the DC combiner box. The DC combiner boxes shall be located such that conduit runs are minimized in the pathways between arrays. DC wiring shall be installed in metallic conduit or raceways when located within enclosed spaces in a building. Conduit shall run along the bottom of load bearing members.

SECTION R325
MEZZANINES

R325.1 General. Mezzanines shall comply with Section R325.

R325.2 Mezzanines. The clear height above and below mezzanine floor construction shall be not less than 7 feet (2134 mm).

R325.3 Area limitation. The aggregate area of a mezzanine or mezzanines shall be not greater than one-third of the floor area of the room or space in which they are located. The enclosed portion of a room shall not be included in a determination of the floor area of the room in which the mezzanine is located.
FOUNdations

FOUNdATION OF HEATED
STructURE PER
SECtIONS R403.1,
R403.2, AND R403.3.2

INSULATION PLACEMENT FOR FROST-PROTECTED FOOTINGS ADJACENT TO HEATED STRUCTURE

FIGURE R403.3(4)

TABLE R403.4
MINIMUM DEPTH OF CRUSHED STONE FOOTINGS' (D), (inches)

<table>
<thead>
<tr>
<th>NUMBER OF STORIES</th>
<th>UNIFORM WALL LOAD</th>
<th>LOAD-BEARING VALUE OF SOIL (psf)</th>
<th>1500</th>
<th>2000</th>
<th>3000</th>
<th>4000</th>
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<tbody>
<tr>
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<tr>
<td></td>
<td></td>
<td></td>
<td>6</td>
<td>8</td>
<td>10</td>
<td>12</td>
</tr>
<tr>
<td>1-story</td>
<td>1100 pfl</td>
<td></td>
<td>6</td>
<td>4</td>
<td>4</td>
<td>4</td>
</tr>
<tr>
<td>2-story</td>
<td>1800 pfl</td>
<td></td>
<td>8</td>
<td>6</td>
<td>4</td>
<td>4</td>
</tr>
<tr>
<td>3-story</td>
<td>2900 pfl</td>
<td></td>
<td>16</td>
<td>14</td>
<td>12</td>
<td>10</td>
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</tbody>
</table>

4-inch brick veneer over light-frame or 8-inch hollow concrete masonry

<table>
<thead>
<tr>
<th>NUMBER OF STORIES</th>
<th>UNIFORM WALL LOAD</th>
<th>LOAD-BEARING VALUE OF SOIL (psf)</th>
<th>1500</th>
<th>2000</th>
<th>3000</th>
<th>4000</th>
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<td></td>
<td></td>
<td></td>
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<td>4</td>
<td>4</td>
<td>4</td>
</tr>
<tr>
<td>1-story</td>
<td>1500 pfl</td>
<td></td>
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<td>12</td>
<td>10</td>
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</tr>
<tr>
<td>2-story</td>
<td>2700 pfl</td>
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<td>14</td>
<td>12</td>
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<td>8</td>
</tr>
<tr>
<td>3-story</td>
<td>4000 pfl</td>
<td></td>
<td>22</td>
<td>22</td>
<td>20</td>
<td>18</td>
</tr>
</tbody>
</table>

8-inch solid or fully grouted masonry

<table>
<thead>
<tr>
<th>NUMBER OF STORIES</th>
<th>UNIFORM WALL LOAD</th>
<th>LOAD-BEARING VALUE OF SOIL (psf)</th>
<th>1500</th>
<th>2000</th>
<th>3000</th>
<th>4000</th>
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<tr>
<td>1-story</td>
<td>2000 pfl</td>
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<td>4</td>
</tr>
<tr>
<td>2-story</td>
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<tr>
<td>3-story</td>
<td>5300 pfl</td>
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<td>22</td>
<td>22</td>
<td>20</td>
<td>18</td>
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</tbody>
</table>

For SI: 1 inch = 25.4 mm. 1 pfl = 14.6 N/m, 1 pound per square foot = 47.9 N/m².

Linear interpolation of stone depth between wall widths is permitted within each Load-Bearing Value of Soil (psf).
SECTION R404
FOUNDATION AND RETAINING WALLS

R404.1 Concrete and masonry foundation walls. Concrete foundation walls shall be selected and constructed in accordance with the provisions of Section R404.1.3. Masonry foundation walls shall be selected and constructed in accordance with the provisions of Section R404.1.2.

R404.1.1 Design required. Concrete or masonry foundation walls shall be designed in accordance with accepted engineering practice where either of the following conditions exists:

1. Walls are subject to hydrostatic pressure from ground water.
2. Walls supporting more than 48 inches (1219 mm) of unbalanced backfill that do not have permanent lateral support at the top or bottom.

R404.1.2 Design of masonry foundation walls. Masonry foundation walls shall be designed and constructed in accordance with the provisions of this section or in accordance with the provisions of TMS 402/ACI 530/ASCE 5. When TMS 402/ACI 530/ASCE 5 or the provisions of this section are used to design masonry foundation walls, project drawings, typical details and specifications are not required to bear the seal of the architect or engineer responsible for design, unless otherwise required by the state law of the jurisdiction having authority.

R404.1.2.1 Masonry foundation walls. Concrete masonry and clay masonry foundation walls shall be constructed as set forth in Table R404.1.1(1), R404.1.1(2), R404.1.1(3) or R404.1.1(4) and shall also comply with applicable provisions of Section R606. In buildings assigned to Seismic Design Categories D₀, D₁, and D₂, concrete masonry and clay masonry foundation walls shall also comply with Section R404.1.4.1. Rubble stone masonry foundation walls shall be constructed in accordance with Sections R404.1.8 and R606.4.2. Rubble stone masonry walls shall not be used in Seismic Design Categories D₀, D₁, and D₂.

R404.1.3 Concrete foundation walls. Concrete foundation walls that support light-frame walls shall be designed and constructed in accordance with the provisions of this section, ACI 318, ACI 332 or PCA 100. Concrete foundation walls that support above-grade concrete walls that are within the applicability limits of Section R608.2 shall be designed and constructed in accordance with the provisions of this section, ACI 318, ACI 332 or PCA 100. Concrete foundation walls that support above-grade concrete walls that are not within the applicability limits of Section R608.2 shall be designed and constructed in accordance with the provisions of ACI 318, ACI 332 or PCA 100. When ACI 318, ACI 332, PCA 100 or the provisions of this section are used to design concrete foundation walls, project drawings, typical details and specifications are not required to bear the seal of the architect or engineer responsible for design, unless otherwise required by the state law of the jurisdiction having authority.
For SI: 1 inch = 25.4 mm.

FIGURE 507.2.3(1)
DECK ATTACHMENT FOR LATERAL LOADS

NOTE:
THIS DETAIL IS APPLICABLE WHERE FLOOR JOISTS ARE PARALLEL TO DECK JOISTS.

FIGURE R507.2.3(2)
DECK ATTACHMENT FOR LATERAL LOADS

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.
R507.7 Deck joist and deck beam bearing. The ends of each joist and beam shall have not less than 1½ inches (38 mm) of bearing on wood or metal and not less than 3 inches (76 mm) on concrete or masonry for the entire width of the beam. Joist framing into the side of a ledger board or beam shall be supported by approved joist hangers. Joists bearing on a beam shall be connected to the beam to resist lateral displacement.

R507.1 Deck post to deck beam. Deck beams shall be attached to deck posts in accordance with Figure R507.7.1 or by other equivalent means capable to resist lateral displacement. Manufactured post-to-beam connectors shall be sized for the post and beam sizes. All bolts shall have washers under the head and nut.

Exception: Where deck beams bear directly on footings in accordance with Section R507.8.1.

### TABLE R507.4
**MAXIMUM JOIST SPACING**

<table>
<thead>
<tr>
<th>MATERIAL TYPE AND NOMINAL SIZE</th>
<th>MAXIMUM ON-CENTER JOIST SPACING</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Perpendicular to joist</td>
</tr>
<tr>
<td>1½-inch-thick wood</td>
<td>16 inches</td>
</tr>
<tr>
<td>2-inch-thick wood</td>
<td>24 inches</td>
</tr>
<tr>
<td>Plastic composite</td>
<td>In accordance with Section R507.3</td>
</tr>
</tbody>
</table>

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 degree = 0.01745 rad.
a. Maximum angle of 45 degrees from perpendicular for wood deck boards

### TABLE R507.5
**DECK JOIST SPANS FOR COMMON LUMBER SPECIES (ft. - in.)**

<table>
<thead>
<tr>
<th>SPECIES†</th>
<th>SIZE</th>
<th>SPACING OF DECK JOISTS WITH NO CANTILEVER‡</th>
<th>SPACING OF DECK JOISTS WITH CANTILEVERS§</th>
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<tr>
<td></td>
<td></td>
<td>(inches)</td>
<td>(inches)</td>
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<tr>
<td></td>
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<td>12</td>
<td>16</td>
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<tr>
<td>Southern pine</td>
<td>2 × 6</td>
<td>9-11</td>
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<td></td>
<td>2 × 8</td>
<td>13-1</td>
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<td></td>
<td>2 × 10</td>
<td>16-2</td>
<td>14-0</td>
</tr>
<tr>
<td></td>
<td>2 × 12</td>
<td>18-0</td>
<td>16-6</td>
</tr>
<tr>
<td>Douglas fir-larch, hem-fir, spruce-pine-fir</td>
<td>2 × 6</td>
<td>9-6</td>
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For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound per square foot = 0.0479 kPa, 1 pound = 0.454 kg.
a. No. 2 grade with wet service factor.
b. Ground snow load, live load = 40 psf, dead load = 10 psf, L/Δ = 360.
c. Ground snow load, live load = 40 psf, dead load = 10 psf, L/Δ = 360 at main span, L/Δ = 180 at cantilever with a 220-pound point load applied to end.
d. Includes incising factor.
e. Northern species with no incising factor.
f. Cantilevered spans not exceeding the nominal depth of the joist are permitted.
NOTE: IF THE DIAGONAL WALL IS GREATER THAN 8 FEET LONG, THEN IT MUST BE TREATED AS A SEPARATE BRACED WALL LINE.

For SI: 1 foot = 304.8 mm.

FIGURE R602.10.1.4
ANGLED WALLS

NOTE: CONTINUOUS SHEATHING METHODS REQUIRE ALL FRAMED PORTIONS OF THE BRACED WALL LINE TO BE SHEATHED.

For SI: 1 foot = 304.8 mm.

FIGURE R602.10.2.2
LOCATION OF BRACED WALL PANELS
### TABLE R602.10.3(1)
**BRACING REQUIREMENTS BASED ON WIND SPEED**

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(continued)
### TABLE R602.10.3(3)—continued
**BRACING REQUIREMENTS BASED ON SEISMIC DESIGN CATEGORY**

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<th>Seismic Design Category</th>
<th>Story Location</th>
<th>Braced Wall Line Length (feet)</th>
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For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound per square foot = 0.0479 kPa.

- **a.** Linear interpolation shall be permitted.
- **b.** Wall bracing lengths are based on a soil site class “D.” Interpolation of bracing length between the $S_a$ values associated with the seismic design categories shall be permitted when a site-specific $S_a$ value is determined in accordance with Section 1613.3 of the California Building Code.
- **c.** Where the braced wall line length is greater than 50 feet, braced wall lines shall be permitted to be divided into shorter segments having lengths of 50 feet or less, and the amount of bracing within each segment shall be in accordance with this table.
- **d.** Method LIB shall have gypsum board fastened to not less than one side with nails or screws in accordance with Table R602.3(1) for exterior sheathing or Table R702.3.3 for interior gypsum board. Spacing of fasteners at panel edges shall not exceed 8 inches.
- **e.** Method CS-SFB does not apply in Seismic Design Categories $D_1$, $D_2$, and $D_3$. 2016 CALIFORNIA RESIDENTIAL CODE SEPTEMBER 1, 2017 ERRATA BUFF 235
### TABLE R602.10.3(4)
SEISMIC ADJUSTMENT FACTORS TO THE REQUIRED LENGTH OF WALL BRACING

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<th>ITEM NUMBER</th>
<th>ADJUSTMENT BASED ON:</th>
<th>STORY</th>
<th>CONDITION</th>
<th>ADJUSTMENT FACTOR&lt;sup&gt;a,b&lt;/sup&gt; [Multiply length from Table R602.10.3(3) by this factor]</th>
<th>APPLICABLE METHODS</th>
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<td>&gt; 10 feet and ≤ 12 feet</td>
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<td>Braced wall line spacing, townhouses in SDC C</td>
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<td>2- or 3-story building</td>
<td>&gt; 15 psf and ≤ 25 psf</td>
<td>1.1</td>
<td></td>
</tr>
<tr>
<td></td>
<td></td>
<td>1-story building</td>
<td>&gt; 15 psf and ≤ 25 psf</td>
<td>1.2</td>
<td></td>
</tr>
<tr>
<td>6</td>
<td>Walls with stone or masonry veneer, townhouses in SDC C&lt;sub&gt;d&lt;/sub&gt;&lt;sup&gt;e&lt;/sup&gt;</td>
<td></td>
<td></td>
<td>1.0</td>
<td></td>
</tr>
<tr>
<td></td>
<td></td>
<td></td>
<td></td>
<td>1.5</td>
<td></td>
</tr>
<tr>
<td></td>
<td></td>
<td></td>
<td></td>
<td>1.5</td>
<td></td>
</tr>
<tr>
<td>7</td>
<td>Walls with stone or masonry veneer, detached one- and two-family dwellings in SDC D&lt;sub&gt;1&lt;/sub&gt;, D&lt;sub&gt;2&lt;/sub&gt;&lt;sup&gt;f&lt;/sup&gt;</td>
<td>Any story</td>
<td>See Table R602.10.6.5</td>
<td>BV-WSP</td>
<td></td>
</tr>
<tr>
<td>8</td>
<td>Interior gypsum board finish (or equivalent)</td>
<td>Any story</td>
<td>Omitted from inside face of braced wall panels</td>
<td>1.5</td>
<td>DWB, WSP, SFB, PBS, PCP, HPS, CS-WSP, CS-G, CS-SFB</td>
</tr>
</tbody>
</table>

For SI: 1 foot = 0.3048 m, 1 pound per square foot = 0.0479 kPa.

a. Linear interpolation shall be permitted.
b. The total length of bracing required for a given wall line is the product of all applicable adjustment factors.
c. The length-to-width ratio for the floor/roof diaphragm shall not exceed 3:1. The top plate lap splice nailing shall be in accordance with Table R602.3(1), Item 13.
d. Applies to stone or masonry veneer exceeding the first story height.
e. The adjustment factor for stone or masonry veneer shall be applied to all exterior braced wall lines and all braced wall lines on the interior of the building, backing or perpendicular to and laterally supported veneered walls.
f. See Section R602.10.6.5 for requirements where stone or masonry veneer does not exceed the first-story height.
R702.3.2 **Wood framing.** Wood framing supporting gypsum board and gypsum panel products shall be not less than 2 inches (51 mm) nominal thickness in the least dimension except that wood furring strips not less than 1-inch by 2-inch (25 mm by 51 mm) nominal dimension shall be permitted to be used over solid backing or framing spaced not more than 24 inches (610 mm) on center.

R702.3.3 **Cold-formed steel framing.** Cold-formed steel framing supporting gypsum board and gypsum panel products shall be not less than 1\(\frac{1}{2}\) inches (32 mm) wide in the least dimension. Nonload-bearing cold-formed steel framing shall comply with AISI S220 and ASTM C645, Section 10. Load-bearing cold-formed steel framing shall comply with AISI S200 and ASTM C955, Section 8.

R702.3.4 **Insulating concrete form walls.** Foam plastics for insulating concrete form walls constructed in accordance with Sections R404.1.2 and R608 on the interior of habitable spaces shall be protected in accordance with Section R316.4. Use of adhesives in conjunction with

### TABLE R702.3.5

<table>
<thead>
<tr>
<th>THICKNESS OF GYPSUM BOARD OR GYPSUM PANEL PRODUCTS (inches)</th>
<th>APPLICATION</th>
<th>ORIENTATION OF GYPSUM BOARD OR GYPSUM PANEL PRODUCTS TO FRAMING</th>
<th>MAXIMUM SPACING OF FRAMING MEMBERS (inches o.c.)</th>
<th>MAXIMUM SPACING OF FASTENERS (inches)</th>
<th>SIZE OF NAILS FOR APPLICATION TO WOOD FRAMING*</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Ceiling</td>
<td>Perpendicular</td>
<td>16</td>
<td>7</td>
<td>12</td>
</tr>
<tr>
<td></td>
<td>Wall</td>
<td>Either direction</td>
<td>16</td>
<td>8</td>
<td>16</td>
</tr>
<tr>
<td>1/4</td>
<td>Ceiling</td>
<td>Either direction</td>
<td>16</td>
<td>7</td>
<td>12</td>
</tr>
<tr>
<td></td>
<td>Ceiling</td>
<td>Perpendicular</td>
<td>24</td>
<td>7</td>
<td>12</td>
</tr>
<tr>
<td></td>
<td>Wall</td>
<td>Either direction</td>
<td>24</td>
<td>8</td>
<td>12</td>
</tr>
<tr>
<td></td>
<td>Wall</td>
<td>Either direction</td>
<td>16</td>
<td>8</td>
<td>16</td>
</tr>
<tr>
<td>1/2</td>
<td>Ceiling</td>
<td>Either direction</td>
<td>16</td>
<td>7</td>
<td>12</td>
</tr>
<tr>
<td></td>
<td>Ceiling</td>
<td>Perpendicular</td>
<td>24</td>
<td>7</td>
<td>12</td>
</tr>
<tr>
<td></td>
<td>Type X at garage ceiling beneath habitable rooms</td>
<td>Perpendicular</td>
<td>24</td>
<td>6</td>
<td>6</td>
</tr>
<tr>
<td></td>
<td>Wall</td>
<td>Either direction</td>
<td>24</td>
<td>8</td>
<td>12</td>
</tr>
<tr>
<td></td>
<td>Wall</td>
<td>Either direction</td>
<td>16</td>
<td>8</td>
<td>16</td>
</tr>
<tr>
<td>5/8</td>
<td>Ceiling</td>
<td>Perpendicular</td>
<td>16</td>
<td>16</td>
<td>16</td>
</tr>
<tr>
<td></td>
<td>Wall</td>
<td>Either direction</td>
<td>16</td>
<td>16</td>
<td>24</td>
</tr>
<tr>
<td>1/2 or 5/8</td>
<td>Ceiling</td>
<td>Either direction</td>
<td>16</td>
<td>16</td>
<td>16</td>
</tr>
<tr>
<td></td>
<td>Ceiling</td>
<td>Perpendicular</td>
<td>24</td>
<td>12</td>
<td>16</td>
</tr>
<tr>
<td></td>
<td>Wall</td>
<td>Either direction</td>
<td>24</td>
<td>16</td>
<td>24</td>
</tr>
<tr>
<td>Two 1/4 layers</td>
<td>Ceiling</td>
<td>Perpendicular</td>
<td>16</td>
<td>16</td>
<td>16</td>
</tr>
<tr>
<td></td>
<td>Wall</td>
<td>Either direction</td>
<td>24</td>
<td>24</td>
<td>24</td>
</tr>
</tbody>
</table>

*For SL: 1 inch = 25.4 mm.

**Note:**
- a. For application without adhesive, a pair of nails spaced not less than 2 inches apart or more than 2\(\frac{1}{2}\) inches apart shall be permitted to be used with the pair of nails spaced 12 inches on center.
- b. Screws shall be in accordance with Section R702.3.5.1. Screws for attaching gypsum board or gypsum panel products to structural insulated panels shall penetrate the wood structural panel facing not less than 1/4 inch.
- c. Where cold-formed steel framing is used with a clinching design to receive nails by two edges of metal, the nails shall be not less than \(\frac{1}{2}\) inch longer than the gypsum board or gypsum panel product thickness and shall have ringed shanks. Where the cold-formed steel framing has a nailing groove formed to receive the nails, the nails shall have barbed shanks or be 5d, 13\(\frac{1}{4}\) gage, 1\(\frac{1}{4}\) inches long, \(\frac{1}{4}\) inches head for 1/4-inch gypsum board or gypsum panel product; and 6d, 13 gage, 1\(\frac{1}{4}\) inches long, \(\frac{1}{4}\) inch head for 1/4-inch gypsum board or gypsum panel product.
- d. Three-eighths-inch thick single-gypsum board or gypsum panel product shall not be used on a ceiling where a water-based textured finish is to be applied, or where it will be required to support insulation above a ceiling. On ceiling applications to receive a water-based texture material, either hand or spray applied, the gypsum board or gypsum panel product shall be applied perpendicular to framing. Where applying a water-based texture material, the minimum gypsum board thickness shall be increased from 1/4 inch to 1/2 inch for 16-inch on center framing, and from 1/2 inch to 1/2 inch for 24-inch on center framing or 1/2-inch sag-resistant gypsum ceiling board shall be used.

For complete code, see pages 171-177 of the 2016 California Residential Code.
mechanical fasteners is permitted. Adhesives used for interior and exterior finishes shall be compatible with the insulating form materials.

**R702.3.5 Application.** Supports and fasteners used to attach gypsum board and gypsum panel products shall comply with Table R702.3.5. Gypsum sheathing shall be attached to exterior walls in accordance with Table R602.3(1). Gypsum board and gypsum panel products shall be applied at right angles or parallel to framing members. All edges and ends of gypsum board and gypsum panel products shall occur on the framing members, except those edges and ends that are perpendicular to the framing members. Interior gypsum board shall not be installed where it is directly exposed to the weather or to water.

**R702.3.5.1 Screw fastening.** Screws for attaching gypsum board and gypsum panel products to wood framing shall be Type W or Type S in accordance with ASTM C1002 and shall penetrate the wood not less than 3/8 inch (15.9 mm). Gypsum board and gypsum panel products shall be attached to cold-formed steel framing with minimum No. 6 screws. Screws for attaching gypsum board and gypsum panel products to cold-formed steel framing less than 0.033 inch (1 mm) thick shall be Type S in accordance with ASTM C1002 or bugle head style in accordance with ASTM C1513 and shall penetrate the steel not less than 3/8 inch (9.5 mm). Screws for attaching gypsum board and gypsum panel products to cold-formed steel framing 0.033 inch to 0.112 inch (1 mm to 3 mm) thick shall be in accordance with ASTM C954 or bugle head style in accordance with ASTM C1513. Screws for attaching gypsum board and gypsum panel products to structural insulated panels shall penetrate the structural panel facing not less than 3/8 inch (11.1 mm).

**R702.3.6 Horizontal gypsum board diaphragm ceilings.** Gypsum board and gypsum panel products shall be permitted on wood joists to create a horizontal diaphragm in accordance with Table R702.3.6. Gypsum board and gypsum panel products shall be installed perpendicular to ceiling framing members. End joints of adjacent courses of board and panels shall not occur on the same joist. The maximum allowable diaphragm proportions shall be 1 1/2:1 between shear resisting elements. Rotation or cantilever conditions shall not be permitted. Gypsum board or gypsum panel products shall not be used in diaphragm ceilings to resist lateral forces imposed by masonry or concrete construction. Perimeter edges shall be blocked using wood members not less than 2-inch by 6-inch (51 mm by 152 mm) nominal dimension. Blocking material shall be installed flat over the top plate of the wall to provide a nailing surface not less than 2 inches (51 mm) in width for the attachment of the gypsum board or gypsum panel product.

**R702.3.7 Water-resistant gypsum backing board.** Gypsum board used as the base or backer for adhesive application of ceramic tile or other required nonabsorbent finish material shall conform to ASTM C1396, C1178 or C1278. Use of water-resistant gypsum backing board shall be permitted on ceilings. Water-resistant gypsum board shall not be installed over a Class I or II vapor retarder in a shower or tub compartment. Cut or exposed edges, including those at wall intersections, shall be sealed as recommended by the manufacturer.

**R702.3.7.1 Limitations.** Water-resistant gypsum backing board shall not be used where there will be direct exposure to water, or in areas subject to continuous high humidity.

**R702.4 Ceramic tile.**

**R702.4.1 General.** Ceramic tile surfaces shall be installed in accordance with ANSI A108.1, A108.4, A108.5, A108.6, A108.11, A118.1, A118.3, A136.1 and A137.1.

**R702.4.2 Backer boards.** Materials used as backers for wall tile in tub and shower areas and wall panels in shower areas shall be of materials listed in Table R702.4.2, and installed in accordance with the manufacturer’s recommendations.

<p>| TABLE R702.3.6 |
|-----------------|-----------------|-----------------|-----------------|-----------------|</p>
<table>
<thead>
<tr>
<th><strong>MATERIAL</strong></th>
<th><strong>THICKNESS OF MATERIAL</strong></th>
<th><strong>SPACING OF FRAMING MEMBERS</strong></th>
<th><strong>SHEAR VALUE</strong>&lt;sup&gt;a,b&lt;/sup&gt; (plf of ceiling)</th>
<th><strong>MINIMUM FASTENER SIZE</strong>&lt;sup&gt;c,d&lt;/sup&gt;</th>
</tr>
</thead>
<tbody>
<tr>
<td>Gypsum board or gypsum panel product</td>
<td>1/2</td>
<td>16 o.c.</td>
<td>90</td>
<td>5d cooler or wallboard nail; 1/4-inch long; 0.086-inch Shank; 1/2-inch head</td>
</tr>
<tr>
<td>Gypsum board or gypsum panel product</td>
<td>1/2</td>
<td>24 o.c.</td>
<td>70</td>
<td>5d cooler or wallboard nail; 1/4-inch long; 0.086-inch Shank; 1/2-inch head</td>
</tr>
</tbody>
</table>

For SI: 1 inch = 25.4 mm, 1 pound per linear foot = 1.488 kg/m.

<sup>a</sup> Values are not cumulative with other horizontal diaphragm values and are for short-term loading caused by wind or seismic loading. Values shall be reduced 25 percent for normal loading.

<sup>b</sup> Values shall be reduced 50 percent in Seismic Design Categories D<sub>5</sub>, D<sub>1</sub>, D<sub>2</sub>, and E.

<sup>c</sup> 1/4-inch, No. 6 Type S or W screws shall be permitted to be substituted for the listed nails.

<sup>d</sup> Fasteners shall be spaced not more than 7 inches on center at all supports, including perimeter blocking, and not less than 1/8 inch from the edges and ends of the gypsum board.
wood framing shall be 1 1/4 inches (32 mm) using minimum 0.120-inch-diameter (3 mm) nail (shank) with a minimum 0.313-inch-diameter head, 16 inches (406 mm) on center. The foam plastic sheathing shall be minimum 1/2-inch-thick (12.7 mm) (nominal) extruded polystyrene in accordance with ASTM C578, 1/2-inch-thick (12.7 mm) (nominal) polyisocyanurate in accordance with ASTM C1289 or 1-inch-thick (25 mm) (nominal) expanded polystyrene in accordance with ASTM C578.

R703.11.2.2 Basic wind speed exceeding 115 miles per hour or Exposure Categories C and D. Where the ultimate design wind speed exceeds 115 miles per hour (51 m/s), the exposure category is C or D, or all conditions of Section R703.11.2.1 are not met, the adjusted design pressure rating for the assembly shall meet or exceed the loads listed in Table R301.2(2) adjusted for height and exposure using Table R301.2(3). The design wind pressure rating of the vinyl siding for installation over solid sheathing as provided in the vinyl siding manufacturer’s product specifications shall be adjusted for the following wall assembly conditions:

1. For wall assemblies with foam plastic sheathing on the exterior side and gypsum wall board, gypsum panel product or equivalent on the interior side of the wall, the vinyl siding’s design wind pressure rating shall be multiplied by 0.39.

2. For wall assemblies with foam plastic sheathing on the exterior side and without gypsum wall board, gypsum panel product or equivalent on the interior side of wall, the vinyl siding’s design wind pressure rating shall be multiplied by 0.27.

R703.11.2.3 Manufacturer specification. Where the vinyl siding manufacturer’s product specifications provide an approved design wind pressure rating for installation over foam plastic sheathing, use of this design wind pressure rating shall be permitted and the siding shall be installed in accordance with the manufacturer’s instructions.

R703.12 Adhered masonry veneer installation. Adhered masonry veneer shall comply with the requirements of Section R703.7.3 and the requirements in Sections 12.1 and 12.3 of TMS 402/ACI 530/ASCE 5. Adhered masonry veneer shall be installed in accordance with Section R703.7.1, Article 3.3C of TMS 602/ACI 530.1/ASCE 6 or the manufacturer’s instructions.

R703.12.1 Clearances. On exterior stud walls, adhered masonry veneer shall be installed:

1. Minimum of 4 inches (102 mm) above the earth;
2. Minimum of 2 inches (51 mm) above paved areas; or
3. Minimum of 1 1/4 inch (12.7 mm) above exterior walking surfaces that are supported by the same foundation that supports the exterior wall.

R703.12.2 Flashing at foundation. A corrosion-resistant screed or flashing of a minimum 0.019-inch (0.48 mm) or 26-gage galvanized or plastic with a minimum vertical attachment flange of 3/8 inches (89 mm) shall be installed to extend a minimum of 1 inch (25 mm) below the founda-
tion plate line on exterior stud walls in accordance with Section R703.4.

R703.12.3 Water-resistive barrier. A water-resistive barrier shall be installed as required by Section R703.2 and shall comply with the requirements of Section R703.7.3. The water-resistive barrier shall lap over the exterior of the attachment flange of the screwed or flashing provided in accordance with Section R703.12.2.

R703.13 Insulated vinyl siding. Insulated vinyl siding shall be certified and labeled as conforming to the requirements of ASTM D7793 by an approved quality control agency.

R703.13.1 Insulated vinyl siding and accessories. Insulated vinyl siding and accessories shall be installed in accordance with manufacturer’s instructions.

R703.14 Polypropylene siding. Polypropylene siding shall be certified and labeled as conforming to the requirements of ASTM D7254 by an approved quality control agency.

R703.14.1 Polypropylene siding and accessories. Polypropylene siding and accessories shall be installed in accordance with manufacturer’s installation instructions.

R703.14.1.1 Installation. Polypropylene siding shall be installed over and attached to wood structural panel sheathing with minimum thickness of 7/16 inch (11.1 mm), or other substrate, composed of wood or wood-based material and fasteners having equivalent withdrawal resistance.

R703.14.1.2 Fastener requirements. Unless otherwise specified in the approved manufacturer’s instructions, nails shall be corrosion resistant, with a minimum 0.120-inch (3 mm) shank and minimum 0.313-inch (8 mm) head diameter. Nails shall be a minimum of 1 1/4 inches (32 mm) long or as necessary to penetrate sheathing or substrate not less than 3/4 inch (19.1 mm). Where the nail fully penetrates the sheathing or nailable substrate, the end of the fastener shall extend not less than 3/4 inch (6.4 mm) beyond the opposite face of the sheathing or substrate. Staples are not permitted.

R703.14.2 Fire separation. Polypropylene siding shall not be installed on walls with a fire separation distance of less than 5 feet (1524 mm) and walls closer than 10 feet (3048 mm) to a building on another lot.

Exception: Walls perpendicular to the line used to determine the fire separation distance.

R703.15 Cladding attachment over foam sheathing to wood framing. Cladding shall be specified and installed in accordance with Section R703, the cladding manufacturer’s approved instructions, including any limitations for use over foam plastic sheathing, or an approved design. In addition, the cladding or furring attachments through foam sheathing to framing shall meet or exceed the minimum fastening requirements of Section R703.15.1, Section R703.15.2, or an approved design for support of cladding weight.

Exceptions:

1. Where the cladding manufacturer has provided approved installation instructions for application over foam sheathing, those requirements shall apply.
2. For exterior insulation and finish systems, refer to Section R703.9.

3. For anchored masonry or stone veneer installed over foam sheathing, refer to Section R703.8.

**R703.15.1 Direct attachment.** Where cladding is installed directly over foam sheathing without the use of furring, cladding minimum fastening requirements to support the cladding weight shall be as specified in Table R703.15.1.

**R703.15.2 Furred cladding attachment.** Where wood furring is used to attach cladding over foam sheathing, furring minimum fastening requirements to support the cladding weight shall be as specified in Table R703.15.2. Where placed horizontally, wood furring shall be preservative-treated wood in accordance with Section R317.1 or naturally durable wood and fasteners shall be corrosion resistant in accordance with Section R317.3.

**R703.16 Cladding attachment over foam sheathing to cold-formed steel framing.** Cladding shall be specified and installed in accordance with Section R703, the cladding manufacturer's approved instructions, including any limitations for use over foam plastic sheathing, or an approved design. In addition, the cladding or furring attachments through foam sheathing to framing shall meet or exceed the minimum fastening requirements of Section R703.16.1, Section R703.16.2 or an approved design for support of cladding weight.

**Exceptions:**

1. Where the cladding manufacturer has provided approved installation instructions for application over foam sheathing, those requirements shall apply.

2. For exterior insulation and finish systems, refer to Section R703.9.

3. For anchored masonry or stone veneer installed over foam sheathing, refer to Section R703.8.

**R703.16.1 Direct attachment.** Where cladding is installed directly over foam sheathing without the use of furring, cladding minimum fastening requirements to support the cladding weight shall be as specified in Table R703.16.1.

**R703.16.2 Furred cladding attachment.** Where steel or wood furring is used to attach cladding over foam sheathing, furring minimum fastening requirements to support the cladding weight shall be as specified in Table R703.16.2. Where placed horizontally, wood furring shall be preservative-treated wood in accordance with Section R317.1 or naturally durable wood and fasteners shall be corrosion resistant in accordance with Section R317.3. Steel furring shall have a minimum G60 galvanized coating.

**R703.17 Cladding attachment over foam sheathing to masonry or concrete wall construction.** Cladding shall be specified and installed in accordance with Section 703.3 and the cladding manufacturer’s instructions or an approved design. Foam sheathing shall be attached to masonry or concrete construction in accordance with the insulation manufacturer’s installation instructions or an approved design. Furring and furring attachments through foam sheathing into concrete or masonry substrate shall be designed to resist design loads determined in accordance with Section R301, including support of cladding weight as applicable. Fasteners used to attach cladding or furring through foam sheathing to masonry or concrete substrates shall be approved for application into masonry or concrete material and shall be installed in accordance with the fastener manufacturer's instructions.

**Exceptions:**

1. Where the cladding manufacturer has provided approved installation instructions for application over foam sheathing and connection to a masonry or concrete substrate, those requirements shall apply.

2. For exterior insulation and finish systems, refer to Section R703.9.

3. For anchored masonry or stone veneer installed over foam sheathing, refer to Section R703.8.
TABLE R802.5.1(1)—continued
RAFTER SPANS FOR COMMON LUMBER SPECIES
(Roof live load=20 psf, ceiling not attached to rafters, L/Δ = 180)

<table>
<thead>
<tr>
<th>RAFTER SPACING (inches)</th>
<th>SPECIES AND GRADE</th>
<th>DEAD LOAD = 10 psf</th>
<th>DEAD LOAD = 20 psf</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td></td>
<td>2 x 4</td>
<td>2 x 6</td>
</tr>
<tr>
<td>Douglas fir-larch SS</td>
<td>9-1</td>
<td>14-4</td>
<td>18-10</td>
</tr>
<tr>
<td>Douglas fir-larch #1</td>
<td>8-7</td>
<td>12-6</td>
<td>15-10</td>
</tr>
<tr>
<td>Douglas fir-larch #2</td>
<td>8-2</td>
<td>11-11</td>
<td>15-1</td>
</tr>
<tr>
<td>Douglas fir-larch #3</td>
<td>6-2</td>
<td>9-1</td>
<td>11-6</td>
</tr>
<tr>
<td>Hem-fir SS</td>
<td>8-7</td>
<td>13-6</td>
<td>17-10</td>
</tr>
<tr>
<td>Hem-fir #1</td>
<td>8-5</td>
<td>12-4</td>
<td>15-8</td>
</tr>
<tr>
<td>Hem-fir #2</td>
<td>7-11</td>
<td>11-7</td>
<td>14-8</td>
</tr>
<tr>
<td>Hem-fir #3</td>
<td>6-1</td>
<td>8-10</td>
<td>11-3</td>
</tr>
<tr>
<td>Southern pine SS</td>
<td>8-11</td>
<td>14-1</td>
<td>18-6</td>
</tr>
<tr>
<td>Southern pine #1</td>
<td>8-7</td>
<td>12-9</td>
<td>16-2</td>
</tr>
<tr>
<td>Southern pine #2</td>
<td>7-4</td>
<td>11-0</td>
<td>13-11</td>
</tr>
<tr>
<td>Southern pine #3</td>
<td>5-8</td>
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<td>10-6</td>
</tr>
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<td>Spruce-pine-fir SS</td>
<td>8-5</td>
<td>13-3</td>
<td>17-5</td>
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<td>Spruce-pine-fir #1</td>
<td>8-0</td>
<td>11-9</td>
<td>14-10</td>
</tr>
<tr>
<td>Spruce-pine-fir #2</td>
<td>8-0</td>
<td>11-9</td>
<td>14-10</td>
</tr>
<tr>
<td>Spruce-pine-fir #3</td>
<td>6-1</td>
<td>8-10</td>
<td>11-3</td>
</tr>
</tbody>
</table>

Maximum rafter spans:

Check sources for availability of lumber in lengths greater than 20 feet.
For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 pound per square foot = 0.0479 kPa.

a. The tabulated rafter spans assume that ceiling joists are located at the bottom of the attic space or that some other method of resisting the outward push of the rafters on the bearing walls, such as rafter ties, is provided at that location. Where ceiling joists or rafter ties are located higher in the attic space, the rafter spans shall be multiplied by the following factors:

$$ H_r/H_n $$

<table>
<thead>
<tr>
<th></th>
<th>Rafter Span Adjustment Factor</th>
</tr>
</thead>
<tbody>
<tr>
<td>1/3</td>
<td>0.67</td>
</tr>
<tr>
<td>1/4</td>
<td>0.76</td>
</tr>
<tr>
<td>1/5</td>
<td>0.83</td>
</tr>
<tr>
<td>1/6</td>
<td>0.90</td>
</tr>
<tr>
<td>1/7.5 or less</td>
<td>1.00</td>
</tr>
</tbody>
</table>

where:

- $H_r$ = Height of ceiling joists or rafter ties measured vertically above the top of the rafter support walls.
- $H_n$ = Height of roof ridge measured vertically above the top of the rafter support walls.

b. Span exceeds 26 feet in length.
**TABLE R802.5.1(2) RAFTER SPANS FOR COMMON LUMBER SPECIES**  
(Roof live load=20 psf, ceiling attached to rafters, L/Δ = 240)

<table>
<thead>
<tr>
<th>RAFTER SPACING (inches)</th>
<th>SPECIES AND GRADE</th>
<th>DEAD LOAD = 10 psf</th>
<th>DEAD LOAD = 20 psf</th>
<th><strong>Maximum rafter spans</strong></th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td></td>
<td>(feet - inches)</td>
<td>(feet - inches)</td>
<td>(feet - inches)</td>
</tr>
<tr>
<td></td>
<td></td>
<td>2 x 4</td>
<td>2 x 6</td>
<td>2 x 8</td>
</tr>
<tr>
<td></td>
<td></td>
<td>2 x 10</td>
<td>2 x 12</td>
<td>2 x 4</td>
</tr>
<tr>
<td></td>
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<td>2 x 6</td>
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<td>2 x 10</td>
</tr>
<tr>
<td></td>
<td></td>
<td>2 x 12</td>
<td>2 x 12</td>
<td></td>
</tr>
<tr>
<td>12</td>
<td>Douglas fir-larch</td>
<td>SS 10-5</td>
<td>16-4</td>
<td>21-7 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#1 10-0</td>
<td>15-9</td>
<td>20-10 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#2 9-10</td>
<td>15-6</td>
<td>20-5 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#3 8-9</td>
<td>12-10</td>
<td>16-3 Note b</td>
</tr>
<tr>
<td></td>
<td>Hem-fir</td>
<td>SS 9-10</td>
<td>15-6</td>
<td>20-5 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#1 9-8</td>
<td>15-2</td>
<td>19-11 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#2 9-2</td>
<td>14-5</td>
<td>19-0 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#3 8-7</td>
<td>12-6</td>
<td>15-10 Note b</td>
</tr>
<tr>
<td></td>
<td>Southern pine</td>
<td>SS 10-3</td>
<td>16-1</td>
<td>21-2 Note b</td>
</tr>
<tr>
<td></td>
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<td></td>
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<td>#3 8-0</td>
<td>11-9</td>
<td>14-10 Note b</td>
</tr>
<tr>
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<td>Spruce-pine-fir</td>
<td>SS 9-8</td>
<td>15-2</td>
<td>19-11 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#1 9-5</td>
<td>14-9</td>
<td>19-6 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#2 9-5</td>
<td>14-9</td>
<td>19-6 Note b</td>
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<td></td>
<td></td>
<td>#3 8-7</td>
<td>12-6</td>
<td>15-10 Note b</td>
</tr>
<tr>
<td>16</td>
<td>Douglas fir-larch</td>
<td>SS 9-6</td>
<td>14-11</td>
<td>19-7 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#1 9-1</td>
<td>14-4</td>
<td>18-11 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#2 8-11</td>
<td>14-1</td>
<td>18-5 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#3 7-7</td>
<td>11-1</td>
<td>14-1 Note b</td>
</tr>
<tr>
<td></td>
<td>Hem-fir</td>
<td>SS 8-11</td>
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<td>18-6 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#1 8-9</td>
<td>13-9</td>
<td>18-1 Note b</td>
</tr>
<tr>
<td></td>
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<td>13-9 Note b</td>
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<td></td>
<td>Southern pine</td>
<td>SS 9-4</td>
<td>14-7</td>
<td>19-3 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#1 8-11</td>
<td>14-1</td>
<td>18-6 Note b</td>
</tr>
<tr>
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<td>#2 8-7</td>
<td>13-5</td>
<td>17-1 Note b</td>
</tr>
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<td></td>
<td></td>
<td>#3 6-11</td>
<td>10-2</td>
<td>12-10 Note b</td>
</tr>
<tr>
<td></td>
<td>Spruce-pine-fir</td>
<td>SS 8-9</td>
<td>13-9</td>
<td>18-1 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#1 8-7</td>
<td>13-5</td>
<td>17-9 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#2 8-7</td>
<td>13-5</td>
<td>17-9 Note b</td>
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<td>13-9 Note b</td>
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<td>Douglas fir-larch</td>
<td>SS 8-11</td>
<td>14-0</td>
<td>18-5 Note b</td>
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<tr>
<td></td>
<td></td>
<td>#1 8-7</td>
<td>13-6</td>
<td>17-9 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
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<td>13-3</td>
<td>16-10 Note b</td>
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<td>#3 6-11</td>
<td>10-2</td>
<td>12-10 Note b</td>
</tr>
<tr>
<td></td>
<td>Hem-fir</td>
<td>SS 8-5</td>
<td>13-3</td>
<td>17-5 Note b</td>
</tr>
<tr>
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<td></td>
<td>#1 8-3</td>
<td>12-11</td>
<td>17-1 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#2 7-10</td>
<td>12-4</td>
<td>16-3 Note b</td>
</tr>
<tr>
<td></td>
<td></td>
<td>#3 6-9</td>
<td>9-11</td>
<td>12-7 Note b</td>
</tr>
</tbody>
</table>

(continued)
For SI: 1 inch = 25.4 mm.

**FIGURE R804.1.2**
IN-LINE FRAMING

**TABLE R804.2.3**
LOAD-BEARING COLD-FORMED STEEL ROOF FRAMING MEMBER SIZES AND THICKNESSES

<table>
<thead>
<tr>
<th>MEMBER DESIGNATION</th>
<th>WEB DEPTH (inches)</th>
<th>MINIMUM BASE STEEL THICKNESS mil (inches)</th>
</tr>
</thead>
<tbody>
<tr>
<td>350S162-t</td>
<td>3.5</td>
<td>33 (0.0329), 43 (0.0428), 54 (0.0538)</td>
</tr>
<tr>
<td>550S162-t</td>
<td>5.5</td>
<td>33 (0.0329), 43 (0.0428), 54 (0.0538), 68 (0.0677)</td>
</tr>
<tr>
<td>800S162-t</td>
<td>8</td>
<td>33 (0.0329), 43 (0.0428), 54 (0.0538), 68 (0.0677)</td>
</tr>
<tr>
<td>1000S162-t</td>
<td>10</td>
<td>43 (0.0428), 54 (0.0538), 68 (0.0677)</td>
</tr>
<tr>
<td>1200S162-t</td>
<td>12</td>
<td>43 (0.0428), 54 (0.0538), 68 (0.0677)</td>
</tr>
</tbody>
</table>

For SI: 1 inch = 25.4 mm

*a. The member designation is defined by the first number representing the member depth in hundredths of an inch, the letter "S" representing a stud or joist member, the second number representing the flange width in hundredths of an inch and the letter "t" shall be a number representing the minimum base metal thickness in mils.

4. The web hole width shall be not greater than one-half the member depth, or 2\(\frac{1}{2}\) inches (64 mm).

5. Holes shall have a web hole length not exceeding 4\(\frac{1}{2}\) inches (114 mm).

6. The minimum distance between the edge of the bearing surface and the edge of the web hole shall be not less than 10 inches (254 mm).

Framing members with web holes not conforming to Items 1 through 6 shall be reinforced in accordance with Section R804.2.6.2, patched in accordance with Section R804.2.6.3 or designed in accordance with accepted engineering practices.

**R804.2.6.2 Web hole reinforcing.** Reinforcement of web holes in ceiling joists not conforming to the requirements of Section R804.2.6.1 shall be permitted if the hole is located fully within the center 40 percent
of the span and the depth and length of the hole does not exceed 65 percent of the flat width of the web. The reinforcing shall be a steel plate or C-shaped section with a hole that does not exceed the web hole size limitations of Section R804.2.6.1 for the member being reinforced. The steel reinforcing shall be the same thickness as the receiving member and shall extend not less than 1 inch (25 mm) beyond all edges of the hole. The steel reinforcing shall be fastened to the web of the receiving member with No. 8 screws spaced not greater than 1 inch (25 mm) center-to-center along the edges of the patch with minimum edge distance of \(\frac{3}{4}\) inch (13 mm).

R804.2.6.3 Hole patching. Patching of web holes in roof framing members not conforming to the requirements in Section R804.2.6.1 shall be permitted in accordance with either of the following methods:

1. Framing members shall be replaced or designed in accordance with accepted engineering practices where web holes exceed either of the following size limits:
   1.1. The depth of the hole, measured across the web, exceeds 70 percent of the flat width of the web.
   1.2. The length of the hole measured along the web, exceeds 10 inches (254 mm) or the depth of the web, whichever is greater.

2. Web holes not exceeding the dimensional requirements in Section R804.2.6.3, Item 1, shall be patched with a solid steel plate, stud section or track section in accordance with Figure R804.2.6.3. The steel patch shall, as a minimum, be the same thickness as the receiving member and shall extend not less than 1 inch (25 mm) beyond all edges of the hole. The steel patch shall be fastened to the web of the receiving member with No. 8 screws spaced not greater than 1 inch (25 mm) center-to-center along the edges of the patch with minimum edge distance of \(\frac{3}{4}\) inch (13 mm).

R804.3 Roof construction. Cold-formed steel roof systems constructed in accordance with the provisions of this section shall consist of both ceiling joists and rafters in accordance with Figure R804.3 and fastened in accordance with Table R804.3.

R804.3.1 Ceiling joists. Cold-formed steel ceiling joists shall be in accordance with this section.

R804.3.1.1 Minimum ceiling joist size. Ceiling joist size and thickness shall be determined in accordance with the limits set forth in Tables R804.3.1.1(1) and R804.3.1.1(2). When determining the size of ceiling joists, the lateral support of the top flange shall be classified as unbraced, braced at midspan or braced at third points in accordance with Section R804.3.1.3. Where sheathing material is attached to the top flange of ceiling joists or where the bracing is spaced closer than third point of the joists, the “third point” values from Tables R804.3.1.1(1) and R804.3.1.1(2) shall be used.

Ceiling joists shall have a bearing support length of not less than 1\(\frac{1}{2}\) inches (38 mm) and shall be connected to roof rafters (heel joint) with No. 10 screws in accordance with Figure R804.3.1.1 and Table R804.3.1.1(3).

Where continuous joists are framed across interior bearing supports, the interior bearing supports shall be located within 24 inches (610 mm) of midspan of the ceiling joist, and the individual spans shall not exceed the applicable spans in Tables R804.3.1.1(1) and R804.3.1.1(2). Where the attic is to be used as an occu-
R804.3.2.5 Roof rafter bottom flange bracing. The bottom flanges of roof rafters shall be continuously braced, at a maximum spacing of 8 feet (2440 mm) as measured parallel to the roof rafters, with one of the following members:
1. Minimum 33-mil (0.84 mm) C-shaped member.
2. Minimum 33-mil (0.84 mm) track section.
3. Minimum 1/4''-inch by 33-mil (38 mm by 0.84 mm) steel strap.

The bracing element shall be fastened to the bottom flange of each roof rafter with one No. 8 screw and shall be fastened to blocking with two No. 8 screws. Blocking shall be installed between roof rafters in-line with the continuous bracing at a maximum spacing of 12 feet (3658 mm) measured perpendicular to the roof rafters. The ends of continuous bracing shall be fastened to blocking or anchored to a stable building component with two No. 8 screws.

R804.3.3 Cutting and notching. Flanges and lips of load-bearing, cold-formed steel roof framing members shall not be cut or notched.

R804.3.4 Headers. Roof-ceiling framing above wall openings shall be supported on headers. The allowable spans for headers in load-bearing walls shall not exceed the values set forth in Section R603.6 and Tables R603.6(1) through R603.6(6).

R804.3.5 Framing of openings in roofs and ceilings. Openings in roofs and ceilings shall be framed with header and trimmer joists. Header joist spans shall not exceed 4 feet (1219 mm) in length. Header and trimmer joists shall be fabricated from joist and track members having a minimum size and thickness equivalent to the adjacent ceiling joists or roof rafters and shall be installed in accordance with Figures R804.3.5(1) and R804.3.5(2). Each header joist shall be connected to trimmer joists with not less than four 2-inch by 2-inch (51 by 51 mm) clip angles. Each clip angle shall be fastened to both the header and trimmer joists with four No. 8 screws, evenly spaced, through each leg of the clip angle. The steel thickness of the clip angles shall be not less than that of the ceiling joist or roof rafter. Each track section for a built-up header or trimmer joist shall extend the full length of the joist (continuous).

R804.3.6 Roof trusses. Cold-formed steel trusses shall be designed and installed in accordance with AISI S100, Section D4. In the absence of specific bracing requirements, trusses shall be braced in accordance with accepted industry practices, such as the SBCA Cold-Formed Steel Building Component Safety Information (CFSBCSI) Guide to Good Practice for Handling, Installing & Bracing of Cold-Formed Steel Trusses. Trusses shall be connected to the top track of the load-bearing wall in accordance with Table R804.3, either with two No. 10 screws applied through the flange of the truss or by using a 54-mil (1.37 mm) clip angle with two No. 10 screws in each leg.

R804.3.7 Ceiling and roof diaphragms. Ceiling and roof diaphragms shall be in accordance with this section.

R804.3.7.1 Ceiling diaphragms. At gable endwalls a ceiling diaphragm shall be provided by attaching a minimum 1/2-inch (12.7 mm) gypsum board or a minimum 1/4-inch (9.5 mm) wood structural panel sheathing, that complies with Section R803, to the bottom of ceiling joists or roof trusses and connected to wall framing in accordance with Figures R804.3.7.1(1) and
ROOF-CEILING CONSTRUCTION

R804.3.7.1(2), unless studs are designed as full height without bracing at the ceiling. Flat blocking shall consist of C-shaped or track section with a minimum thickness of 33 mils (0.84 mm). For a gypsum board sheathed ceiling, the diaphragm length shall be in accordance with Table R804.3.7.1. For a wood structural panel sheathed ceiling, the diaphragm length shall be not less than 12 feet (3658 mm) for building widths less than 36 feet (10 973 mm), or not less than 14 feet (4267 mm) for building widths greater than or equal to 36 feet (10 973 mm).

The ceiling diaphragm shall be secured with screws spaced at a maximum 6 inches (152 mm) o.c. at panel edges and a maximum 12 inches (305 mm) o.c. in the field. The required lengths in Table R804.3.7.1 for gypsum board sheathed ceiling diaphragms shall be permitted to be multiplied by 0.35 if all panel edges are blocked. Multiplying the required lengths in Table R804.3.7.1 for gypsum board sheathed ceiling diaphragms by 0.9 shall be permitted if all panel edges are secured with screws spaced at 4 inches (102 mm) o.c.

R804.3.7.2 Roof diaphragm. A roof diaphragm shall be provided by attaching a minimum of $\frac{3}{8}$-inch (9.5 mm) wood structural panel which complies with Section R803 to roof rafters or truss top chords in accordance with Table R804.3. Buildings with 3:1 or larger plan aspect ratio and with roof rafter slope (pitch) of 9:12 or larger shall have the roof rafters and ceiling joists blocked in accordance with Figure R804.3.7.2.

R804.3.8 Roof tie-down. Roof assemblies shall be connected to walls below in accordance with Table R804.3. A continuous load path shall be provided to transfer uplift loads to the foundation.

SECTION R805
CEILING FINISHES

R805.1 Ceiling installation. Ceilings shall be installed in accordance with the requirements for interior wall finishes as provided in Section R702.

SECTION R806
ROOF VENTILATION

R806.1 Ventilation required. Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or snow. Ventilation openings shall have a least dimension of $\frac{1}{4}$ inch (1.6 mm) minimum and $\frac{1}{2}$ inch (6.4 mm) maximum. Ventilation openings having a least dimension larger than $\frac{1}{4}$ inch (6.4 mm) shall be provided with corrosion-resistant wire cloth screening, hardware cloth or similar material with openings having a least dimension of $\frac{1}{4}$ inch (1.6 mm) minimum and $\frac{1}{2}$ inch (6.4 mm) maximum. Openings in roof framing members shall conform to the requirements of Section R802.7. Required ventilation openings shall open directly to the outside air.
TABLE R804.3.7.1
REQUIRED LENGTHS FOR CEILING DIAPHRAGMS AT GABLE ENDWALLS
GYPSUM BOARD SHEATHED, CEILING HEIGHT = 8 FEET

<table>
<thead>
<tr>
<th>EXPOSURE CATEGORY</th>
<th>ULTIMATE DESIGN WIND SPEED (mph)</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>B</td>
</tr>
<tr>
<td>C</td>
<td>110</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Roof pitch</th>
<th>Building endwall width (feet)</th>
<th>Minimum diaphragm length (feet)</th>
</tr>
</thead>
<tbody>
<tr>
<td>3:12 to 6:12</td>
<td>24 - 28</td>
<td>20</td>
</tr>
<tr>
<td></td>
<td>&gt; 28 - 32</td>
<td>22</td>
</tr>
<tr>
<td></td>
<td>&gt; 32 - 36</td>
<td>26</td>
</tr>
<tr>
<td></td>
<td>&gt; 36 - 40</td>
<td>30</td>
</tr>
<tr>
<td>6:12 to 9:12</td>
<td>&gt; 24 - 28</td>
<td>22</td>
</tr>
<tr>
<td></td>
<td>&gt; 28 - 32</td>
<td>26</td>
</tr>
<tr>
<td></td>
<td>&gt; 32 - 36</td>
<td>32</td>
</tr>
<tr>
<td></td>
<td>&gt; 36 - 40</td>
<td>36</td>
</tr>
<tr>
<td>9:12 to 12:12</td>
<td>&gt; 24 - 28</td>
<td>26</td>
</tr>
<tr>
<td></td>
<td>&gt; 28 - 32</td>
<td>30</td>
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<tr>
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<td>&gt; 32 - 36</td>
<td>36</td>
</tr>
<tr>
<td></td>
<td>&gt; 36 - 40</td>
<td>42</td>
</tr>
</tbody>
</table>

For SI: 1 inch = 25.4 mm, 1 pound per square foot = 0.0479 kPa, 1 mile per hour = 0.447 m/s, 1 foot = 304.8 mm, 1 mil = 0.00254 mm.

a. Ceiling diaphragm is composed of 1/2-inch gypsum board (min. thickness) secured with screws spaced at 6 inches o.c. at panel edges and 12 inches o.c. infield.

b. Use No. 8 screws (min.) where framing members have a designation thickness of 54 mils or less and No. 10 screws (min.) where framing members have a designation thickness greater than 54 mils.

c. Maximum aspect ratio (length/width) of diaphragms is 2:1.

d. Building width is in the direction of horizontal framing members supported by the wall studs.

e. Required diaphragm lengths are to be provided at each end of the structure.

For SI: 1 inch = 25.4 mm.

FIGURE R804.3.7.1(1)
CEILING DIAPHRAGM TO GABLE ENDWALL DETAIL
FIGURE R804.3.7.1(2)
CEILING DIAPHRAGM TO SIDEWALL DETAIL

For SI: 1 inch = 25.4 mm.

FIGURE R804.3.7.2
ROOF BLOCKING DETAIL

For SI: 1 mil = 0.0254 mm, 1 inch = 25.4 mm.
R806.2 Minimum vent area. The minimum net free ventilating area shall be \( \frac{1}{160} \) of the area of the vented space.

Exception: The minimum net free ventilation area shall be \( \frac{1}{180} \) of the vented space provided one or more of the following conditions are met:

1. In Climate Zones 14 and 16, a Class I or II vapor retarder is installed on the warm-in-winter side of the ceiling.

2. Not less than 40 percent and not more than 50 percent of the required ventilating area is provided by ventilators located in the upper portion of the attic or rafter space. Upper ventilators shall be located not more than 3 feet (914 mm) below the ridge or highest point of the space, measured vertically, with the balance of the required ventilation provided by eave or cornice vents. Where the location of wall or roof framing members conflicts with the installation of upper ventilators, installation more than 3 feet (914 mm) below the ridge or highest point of the space shall be permitted.

R806.3 Vent and insulation clearance. Where eave or cornice vents are installed, insulation shall not block the free flow of air. Not less than a 1-inch (25 mm) space shall be provided between the insulation and the roof sheathing and at the location of the vent.

R806.4 Installation and weather protection. Ventilators shall be installed in accordance with manufacturer’s instructions. Installation of ventilators in roof systems shall be in accordance with the requirements of Section R903. Installation of ventilators in wall systems shall be in accordance with the requirements of Section R703.1.

R806.5 Unvented attic and unvented enclosed rafter assemblies. Unvented attics and unvented enclosed roof framing assemblies created by ceilings that are applied directly to the underside of the roof framing members and structural roof sheathing applied directly to the top of the roof framing members/rafters, shall be permitted where all the following conditions are met:

1. The unvented attic space is completely within the building thermal envelope.

2. No interior Class I vapor retarders are installed on the ceiling side (attic floor) of the unvented attic assembly or on the ceiling side of the unvented enclosed roof framing assembly.

3. Where wood shingles or shakes are used, a minimum \( \frac{1}{4} \) -inch (6.4 mm) vented airspace separates the shingles or shakes and the roofing underlayment above the structural sheathing.

4. In California Climate Zones 14 and 16, any air-impermeable insulation shall be a Class II vapor retarder, or shall have a Class II vapor retarder coating or covering in direct contact with the underside of the insulation.

4.1. A Class I or Class II vapor retarder shall be installed on the indirectly conditioned space side of all insulation in an unvented attic with air-permeable insulation, for condensation control.

See the California Energy Code, Figure 1001-3A –California Climate Zones.

5. Insulation shall be located in accordance with the following:

5.1. Item 5.1.1, 5.1.2, 5.1.3 or 5.1.4 shall be met, depending on the air permeability of the insulation directly under the structural roof sheathing. No insulation shall be required when roof tiles, wood shingles or wood shakes, or any other roofing system using battens and no continuous underlayment is installed. A continuous underlayment shall be considered to exist if sheathing, roofing paper or any continuous layer having a perm rate of no more than one perm under the dry cup method is present.

5.1.1. Where only air-impermeable insulation is provided, it shall be applied in direct contact with the underside of the structural roof sheathing.

5.1.2. Where air-permeable insulation is provided inside the building thermal envelope, it shall be installed in accordance with Section 5.1.1. In addition to the air-permeable insulation installed directly below the structural sheathing, rigid board or sheet insulation shall be installed directly above the structural roof sheathing in accordance with the R-values in Table R806.5 for condensation control.

5.1.3. Where both air-impermeable and air-permeable insulation are provided, the air-impermeable insulation shall be applied in direct contact with the underside of the structural roof sheathing and shall be in accordance with the R-values in Table R806.5 for condensation control. The air-permeable insulation shall be installed directly under the air-impermeable insulation.

5.1.4. Alternatively, sufficient rigid board or sheet insulation shall be installed directly above the structural roof sheathing to maintain the monthly average temperature of the underside of the structural roof sheathing above 45°F (7°C). For calculation purposes, an interior air temperature of 68°F (20°C) is assumed and the exterior air tempera-
ture is assumed to be the monthly average outside air temperature of the three coldest months.

5.2. Where preformed insulation board is used as the air-impermeable insulation layer, it shall be sealed at the perimeter of each individual sheet interior surface to form a continuous layer.

**TABLE R806.5**
**INSULATION FOR CONDENSATION CONTROL**

<table>
<thead>
<tr>
<th>CLIMATE ZONE</th>
<th>MINIMUM RIGID BOARD OR AIR-IMPERMEABLE INSULATION R-VALUE</th>
</tr>
</thead>
<tbody>
<tr>
<td>6-15 tile roof only</td>
<td>0 (none required)</td>
</tr>
<tr>
<td>3-15</td>
<td>R-5</td>
</tr>
<tr>
<td>1 &amp; 2</td>
<td>R-10</td>
</tr>
<tr>
<td>16</td>
<td>R-15</td>
</tr>
</tbody>
</table>

**SECTION R807**
**ATTIC ACCESS**

**R807.1 Attic access.** Buildings with combustible ceiling or roof construction shall have an attic access opening to attic areas that have a vertical height of 30 inches (762 mm) or greater over an area of not less than 30 square feet (2.8 m²). The vertical height shall be measured from the top of the ceiling framing members to the underside of the roof framing members.

The rough-framed opening shall be not less than 22 inches by 30 inches (559 mm by 762 mm) and shall be located in a hallway or other readily accessible location. Where located in a wall, the opening shall be not less than 22 inches wide by 30 inches high (559 mm wide by 762 mm high). Where the access is located in a ceiling, minimum unobstructed headroom in the attic space shall be 30 inches (762 mm) at some point above the access measured vertically from the bottom of ceiling framing members. See the California Mechanical Code for access requirements where mechanical equipment is located in attics.
TABLE R905.1.1(3)
UNDERLAYMENT ATTACHMENT

<table>
<thead>
<tr>
<th>ROOF COVERING</th>
<th>SECTION</th>
<th>MAXIMUM ULTIMATE DESIGN WIND SPEED, ( V_{aw} &lt; 140 ) MPH</th>
<th>MAXIMUM ULTIMATE DESIGN WIND SPEED, ( V_{aw} \geq 140 ) MPH</th>
</tr>
</thead>
<tbody>
<tr>
<td>Asphalt shingles</td>
<td>R905.2</td>
<td>Fastened sufficiently to hold in place</td>
<td>The underlayment shall be attached with corrosion-resistant fasteners in a grid pattern of 12 inches between side laps with a 6-inch spacing at the side laps. Underlayment shall be attached using metal or plastic cap nails or cap staples with a nominal cap diameter of not less than 1 inch. Metal caps shall have a thickness of not less than 32-gage sheet metal. Power-driven metal caps shall have a minimum thickness of 0.010 inch. Minimum thickness of the outside edge of plastic caps shall be 0.035 inch. The cap nail shank shall be not less than 0.083 inch for ring shank cap nails and 0.091 inch for smooth shank cap nails. Staples shall be not less than 21 gage. Cap nail shank and cap staple legs shall have a length sufficient to penetrate through the roof sheathing or not less than ( \frac{3}{16} ) inch into the roof sheathing.</td>
</tr>
<tr>
<td>Clay and concrete tile</td>
<td>R905.3</td>
<td>Fastened sufficiently to hold in place</td>
<td>The underlayment shall be attached with corrosion-resistant fasteners in a grid pattern of 12 inches between side laps with a 6-inch spacing at the side laps. Underlayment shall be attached using metal or plastic cap nails or cap staples with a nominal cap diameter of not less than 1 inch. Metal caps shall have a thickness of not less than 32-gage sheet metal. Power-driven metal caps shall have a minimum thickness of 0.010 inch. Minimum thickness of the outside edge of plastic caps shall be 0.035 inch. The cap nail shank shall be not less than 0.083 inch for ring shank cap nails and 0.091 inch for smooth shank cap nails. Staples shall be not less than 21 gage. Cap nail shank and cap staple legs shall have a length sufficient to penetrate through the roof sheathing or not less than ( \frac{3}{16} ) inch into the roof sheathing.</td>
</tr>
<tr>
<td>Metal roof shingles</td>
<td>R905.4</td>
<td>Manufacturer's installation instructions.</td>
<td>The underlayment shall be attached with corrosion-resistant fasteners in a grid pattern of 12 inches between side laps with a 6-inch spacing at the side laps. Underlayment shall be attached using metal or plastic cap nails or cap staples with a nominal cap diameter of not less than 1 inch. Metal caps shall have a thickness of at least 32-gage sheet metal. Power-driven metal caps shall have a minimum thickness of 0.010 inch. Minimum thickness of the outside edge of plastic caps shall be 0.035 inch. The cap nail shank shall be not less than 0.083 inch for ring shank cap nails and 0.091 inch for smooth shank cap nails. Staples shall be not less than 21 gage. Cap nail shank and cap staple legs shall have a length sufficient to penetrate through the roof sheathing or not less than ( \frac{3}{16} ) inch into the roof sheathing.</td>
</tr>
<tr>
<td>Mineral-surfaced roll roofing</td>
<td>R905.5</td>
<td>Fastened sufficiently to hold in place</td>
<td>The underlayment shall be attached with corrosion-resistant fasteners in a grid pattern of 12 inches between side laps with a 6-inch spacing at the side laps. Underlayment shall be attached using metal or plastic cap nails or cap staples with a nominal cap diameter of not less than 1 inch. Metal caps shall have a thickness of at least 32-gage sheet metal. Power-driven metal caps shall have a minimum thickness of 0.010 inch. Minimum thickness of the outside edge of plastic caps shall be 0.035 inch. The cap nail shank shall be not less than 0.083 inch for ring shank cap nails and 0.091 inch for smooth shank cap nails. Staples shall be not less than 21 gage. Cap nail shank and cap staple legs shall have a length sufficient to penetrate through the roof sheathing or not less than ( \frac{3}{16} ) inch into the roof sheathing.</td>
</tr>
<tr>
<td>Slate and slate-type shingles</td>
<td>R905.6</td>
<td>Fastened sufficiently to hold in place</td>
<td>The underlayment shall be attached with corrosion-resistant fasteners in a grid pattern of 12 inches between side laps with a 6-inch spacing at the side laps. Underlayment shall be attached using metal or plastic cap nails or cap staples with a nominal cap diameter of not less than 1 inch. Metal caps shall have a thickness of at least 32-gage sheet metal. Power-driven metal caps shall have a minimum thickness of 0.010 inch. Minimum thickness of the outside edge of plastic caps shall be 0.035 inch. The cap nail shank shall be not less than 0.083 inch for ring shank cap nails and 0.091 inch for smooth shank cap nails. Staples shall be not less than 21 gage. Cap nail shank and cap staple legs shall have a length sufficient to penetrate through the roof sheathing or not less than ( \frac{3}{16} ) inch into the roof sheathing.</td>
</tr>
<tr>
<td>Wood shingles</td>
<td>R905.7</td>
<td>Fastened sufficiently to hold in place</td>
<td>The underlayment shall be attached with corrosion-resistant fasteners in a grid pattern of 12 inches between side laps with a 6-inch spacing at the side laps. Underlayment shall be attached using metal or plastic cap nails or cap staples with a nominal cap diameter of not less than 1 inch. Metal caps shall have a thickness of at least 32-gage sheet metal. Power-driven metal caps shall have a minimum thickness of 0.010 inch. Minimum thickness of the outside edge of plastic caps shall be 0.035 inch. The cap nail shank shall be not less than 0.083 inch for ring shank cap nails and 0.091 inch for smooth shank cap nails. Staples shall be not less than 21 gage. Cap nail shank and cap staple legs shall have a length sufficient to penetrate through the roof sheathing or not less than ( \frac{3}{16} ) inch into the roof sheathing.</td>
</tr>
<tr>
<td>Wood shakes</td>
<td>R905.8</td>
<td>Fastened sufficiently to hold in place</td>
<td>The underlayment shall be attached with corrosion-resistant fasteners in a grid pattern of 12 inches between side laps with a 6-inch spacing at the side laps. Underlayment shall be attached using metal or plastic cap nails or cap staples with a nominal cap diameter of not less than 1 inch. Metal caps shall have a thickness of at least 32-gage sheet metal. Power-driven metal caps shall have a minimum thickness of 0.010 inch. Minimum thickness of the outside edge of plastic caps shall be 0.035 inch. The cap nail shank shall be not less than 0.083 inch for ring shank cap nails and 0.091 inch for smooth shank cap nails. Staples shall be not less than 21 gage. Cap nail shank and cap staple legs shall have a length sufficient to penetrate through the roof sheathing or not less than ( \frac{3}{16} ) inch into the roof sheathing.</td>
</tr>
<tr>
<td>Metal panels</td>
<td>R905.10</td>
<td>Fastened sufficiently to hold in place</td>
<td>The underlayment shall be attached with corrosion-resistant fasteners in a grid pattern of 12 inches between side laps with a 6-inch spacing at the side laps. Underlayment shall be attached using metal or plastic cap nails or cap staples with a nominal cap diameter of not less than 1 inch. Metal caps shall have a thickness of at least 32-gage sheet metal. Power-driven metal caps shall have a minimum thickness of 0.010 inch. Minimum thickness of the outside edge of plastic caps shall be 0.035 inch. The cap nail shank shall be not less than 0.083 inch for ring shank cap nails and 0.091 inch for smooth shank cap nails. Staples shall be not less than 21 gage. Cap nail shank and cap staple legs shall have a length sufficient to penetrate through the roof sheathing or not less than ( \frac{3}{16} ) inch into the roof sheathing.</td>
</tr>
</tbody>
</table>

For SI: 1 inch = 25.4 mm.

R905.2.4 Asphalt shingles. Asphalt shingles shall comply with ASTM D3462.

R905.2.4.1 Wind resistance of asphalt shingles. Asphalt shingles shall be tested in accordance with ASTM D7158. Asphalt shingles shall meet the classification requirements of Table R905.2.4.1 for the appropriate ultimate design wind speed. Asphalt shingle packaging shall bear a label to indicate compliance with ASTM D7158 and the required classification in Table R905.2.4.1.

Exception: Asphalt shingles not included in the scope of ASTM D7158 shall be tested and labeled to indicate compliance with ASTM D3161 and the required classification in Table R905.2.4.1.

R905.2.5 Fasteners. Fasteners for asphalt shingles shall be galvanized steel, stainless steel, aluminum or copper roofing nails, minimum 12-gage [0.105 inch (3 mm)] shank with a minimum \( \frac{3}{16} \) inch-diameter (9.5 mm) head, complying with ASTM F1667, of a length to penetrate through the roofing materials and not less than \( \frac{3}{16} \) inch (19.1 mm) into the roof sheathing. Where the roof sheathing is less than \( \frac{3}{16} \) inch (19.1 mm) thick, the fasteners shall penetrate through the sheathing.

R905.2.6 Attachment. Asphalt shingles shall have the minimum number of fasteners required by the manufacturer, but not less than four fasteners per strip shingle or two fasteners per individual shingle. Where the roof slope exceeds 21 units vertical in 12 units horizontal (21:12, 175-percent slope), shingles shall be installed as required by the manufacturer.

R905.2.7 Ice barrier. Where required, ice barriers shall comply with Section R905.1.2.

R905.2.8 Flashing. Flashing for asphalt shingles shall comply with this section.

R905.2.8.1 Base and cap flashing. Base and cap flashing shall be installed in accordance with manufacturer’s instructions. Base flashing shall be of either corrosion-resistant metal of minimum nominal 0.019-inch (0.5 mm) thickness or mineral-surfaced roll roofing weighing not less than 77 pounds per 100 square feet (4 kg/m²). Cap flashing shall be corrosion-resistant metal of minimum nominal 0.019-inch (0.5 mm) thickness.

R905.2.8.2 Valleys. Valley linings shall be installed in accordance with the manufacturer’s instructions before applying shingles. Valley linings of the following types shall be permitted:

1. For open valleys (valley lining exposed) lined with metal, the valley lining shall be not less than
### Table R905.2.4.1
Classification of Asphalt Roof Shingles

<table>
<thead>
<tr>
<th>Maximum Ultimate Design Wind Speed, $V_u$ from Figure R301.2.4(1) (mph)</th>
<th>Maximum Basic Wind Speed, $V_{bas}$ from Table R301.2.1.3 (mph)</th>
<th>ASTM D7158* Shingle Classification</th>
<th>ASTM D3161 Shingle Classification</th>
</tr>
</thead>
<tbody>
<tr>
<td>110</td>
<td>85</td>
<td>D, G or H</td>
<td>A, D or F</td>
</tr>
<tr>
<td>116</td>
<td>90</td>
<td>D, G or H</td>
<td>A, D or F</td>
</tr>
<tr>
<td>129</td>
<td>100</td>
<td>G or H</td>
<td>A, D or F</td>
</tr>
<tr>
<td>142</td>
<td>110</td>
<td>G or H</td>
<td>F</td>
</tr>
<tr>
<td>155</td>
<td>120</td>
<td>G or H</td>
<td>F</td>
</tr>
<tr>
<td>168</td>
<td>130</td>
<td>H</td>
<td>F</td>
</tr>
<tr>
<td>181</td>
<td>140</td>
<td>H</td>
<td>F</td>
</tr>
<tr>
<td>194</td>
<td>150</td>
<td>H</td>
<td>F</td>
</tr>
</tbody>
</table>

For SI: 1 foot = 304.8 mm; 1 mph = 0.447 m/s.

a. The standard calculations contained in ASTM D7158 assume Exposure Category B or C and building height of 60 feet or less. Additional calculations are required for conditions outside of these assumptions.

24 inches (610 mm) wide and of any of the corrosion-resistant metals in Table R905.2.8.2.

2. For open valleys, valley lining of two plies of mineral-surfaced roll roofing, complying with ASTM D3909 or ASTM D6380 Class M, shall be permitted. The bottom layer shall be 18 inches (457 mm) and the top layer not less than 36 inches (914 mm) wide.

3. For closed valleys (valley covered with shingles), valley lining of one ply of smooth roll roofing complying with ASTM D6380 and not less than 36 inches wide (914 mm) or valley lining as described in Item 1 or 2 shall be permitted. Self-adhering polymer modified bitumen underlayment complying with ASTM D1970 shall be permitted in lieu of the lining material.

### R905.2.8.3 Sidewall flashing
Base flashing against a vertical sidewall shall be continuous or step flashing and shall be not less than 4 inches (102 mm) in height and 4 inches (102 mm) in width and shall direct water away from the vertical sidewall onto the roof or into the gutter. Where siding is provided on the vertical sidewall, the vertical leg of the flashing shall be continuous under the siding. Where anchored masonry veneer is provided on the vertical sidewall, the base flashing shall be provided in accordance with this section and counterflashing shall be provided in accordance with Section R703.8.2.2. Where exterior plaster or adhered masonry veneer is provided on the vertical sidewalk, the base flashing shall be provided in accordance with this section and Section R703.6.3.

### R905.2.8.4 Other flashing
Flashing against a vertical front wall, as well as soil stack, vent pipe and chimney flashing, shall be applied in accordance with the asphalt shingle manufacturer’s printed instructions.

### R905.2.8.5 Drip edge
A drip edge shall be provided at eaves and rake edges of shingle roofs. Adjacent segments of drip edge shall be overlapped not less than 2 inches (51 mm). Drip edges shall extend not less than $\frac{3}{4}$ inch (6.4 mm) below the roof sheathing and extend up back onto the roof deck not less than 2 inches (51 mm). Drip edges shall be mechanically fastened to the roof deck at not more than 12 inches (305 mm) o.c. with fasteners as specified in Section R905.2.5. Underlayment shall be installed over the drip edge along eaves and under the drip edge along rake edges.

### R905.3 Clay and concrete tile
The installation of clay and concrete tile shall comply with the provisions of this section.

#### R905.3.1 Deck requirements
Concrete and clay tile shall be installed only over solid sheathing or spaced structural sheathing boards.

#### R905.3.2 Deck slope
Clay and concrete roof tile shall be installed on roof slopes of two and one-half units vertical in 12 units horizontal (2:12:12) or greater. For roof slopes from two and one-half units vertical in 12 units horizontal (2:12:12) to four units vertical in 12 units horizontal (4:12), double underlayment application is required in accordance with Section R905.3.3.

#### R905.3.3 Underlayment
Underlayment shall comply with Section R905.1.1.

#### R905.3.4 Clay tile
Clay roof tile shall comply with ASTM C1167.

#### R905.3.5 Concrete tile
Concrete roof tile shall comply with ASTM C1492.

#### R905.3.6 Fasteners
Nails shall be corrosion resistant and not less than 11 gage, $\frac{3}{8}$-inch (11 mm) head, and of sufficient length to penetrate the deck not less than $\frac{3}{4}$ inch (19 mm) or through the thickness of the deck, whichever is less. Attaching wire for clay or concrete tile shall not be smaller than 0.083 inch (2 mm). Perimeter fastening areas include three tile courses but not less than 36 inches (914 mm) from either side of hips or ridges and edges of eaves and gable rakes.
SECTION R1002
MASONRY HEATERS

R1002.1 Definition. A masonry heater is a heating appliance constructed of concrete or solid masonry, hereinafter referred to as masonry, that is designed to absorb and store heat from a solid-fuel fire built in the firebox by routing the exhaust gases through internal heat exchange channels in which the flow path downstream of the firebox includes flow in a horizontal or downward direction before entering the chimney and that delivers heat by radiation from the masonry surface of the heater.

R1002.2 Installation. Masonry heaters shall be installed in accordance with this section and comply with one of the following:

1. Masonry heaters shall comply with the requirements of ASTM E1602.

2. Masonry heaters shall be listed and labeled in accordance with UL 1482 or CEN 15250 and installed in accordance with the manufacturer’s instructions.

R1002.3 Footings and foundation. The firebox floor of a masonry heater shall be a minimum thickness of 4 inches (102 mm) of noncombustible material and be supported on a noncombustible footing and foundation in accordance with Section R1003.2.

R1002.4 Seismic reinforcing. In Seismic Design Categories D_n, D_l, and D_s, masonry heaters shall be anchored to the masonry foundation in accordance with Section R1003. Seismic reinforcing shall not be required within the body of a masonry heater whose height is equal to or less than 3.5 times its body width and where the masonry chimney serving the heater is not supported by the body of the heater. Where the masonry chimney shares a common wall with the facing of the masonry heater, the chimney portion of the structure shall be reinforced in accordance with Section R1003.

R1002.5 Masonry heater clearance. Combustible materials shall not be placed within 36 inches (914 mm) of the outside surface of a masonry heater in accordance with NFPA 211 Section 8-7 (clearances for solid-fuel-burning appliances), and the required space between the heater and combustible material shall be fully vented to permit the free flow of air around all heater surfaces.

Exceptions:

1. Where the masonry heater wall is not less than 8 inches (203 mm) thick of solid masonry and the wall of the heat exchange channels is not less than 5 inches (127 mm) thick of solid masonry, combustible materials shall not be placed within 4 inches (102 mm) of the outside surface of a masonry heater. A clearance of not less than 8 inches (203 mm) shall be provided between the gas-tight capping slab of the heater and a combustible ceiling.

2. Masonry heaters listed and labeled in accordance with UL 1482 or CEN 15250 shall be installed in accordance with the listing specifications and the manufacturer’s written instructions.

SECTION R1003
MASONRY CHIMNEYS

R1003.1 Definition. A masonry chimney is a chimney constructed of solid masonry units, hollow masonry units grouted solid, stone or concrete, hereinafter referred to as masonry. Masonry chimneys shall be constructed, anchored, supported and reinforced as required in this chapter.

R1003.2 Footings and foundations. Footings for masonry chimneys shall be constructed of concrete or solid masonry not less than 12 inches (305 mm) thick and shall extend not less than 6 inches (152 mm) beyond the face of the foundation or support wall on all sides. Footings shall be founded on natural undisturbed earth or engineered fill below frost depth. In areas not subjected to freezing, footings shall be not less than 12 inches (305 mm) below finished grade.

R1003.3 Seismic reinforcing. Masonry or concrete chimneys shall be constructed, anchored, supported and reinforced as required in this chapter. In all structures regulated by this code assigned to Seismic Design Category C, D_n, D_l or D_s, masonry and concrete chimneys shall be reinforced and
anchored as detailed in Section R1003.3.1, R1003.3.2 and R1003.4. In Seismic Design Category A or B, reinforcement and seismic anchorage is not required.

R1003.3.1 Vertical reinforcing. For chimneys up to 40 inches (1016 mm) wide, four No. 4 continuous vertical bars, anchored in the foundation, shall be placed in the concrete, or between wythes of solid masonry, or within the cells of hollow unit masonry, and grouted in accordance with Section R608.1.1. Grout shall be prevented from bonding with the flue liner so that the flue liner is free to move with thermal expansion. For chimneys more than 40 inches (1016 mm) wide, two additional No. 4 vertical bars shall be installed for each additional 40 inches (1016 mm) in width or fraction thereof.

R1003.3.2 Horizontal reinforcing. Vertical reinforcement shall be placed enclosed within 1/4-inch (6.4 mm) ties, or other reinforcing of equivalent net cross-sectional area, spaced not to exceed 18 inches (457 mm) on center in concrete, or placed in the bed joints of unit masonry, at not less than every 18 inches (457 mm) of vertical height. Two such ties shall be installed at each bend in the vertical bars.

R1003.4 Seismic anchorage. Masonry and concrete chimneys and foundations in all structures regulated by this code assigned to Seismic Design Category C, D, or D, shall be anchored at each floor, ceiling or roof line more than 6 feet (1829 mm) above grade, except where constructed completely within the exterior walls. Anchorage shall conform to the requirements in Section R1003.4.1.

R1003.4.1 Anchorage. Two 1/4-inch by 1-inch (5 mm by 25 mm) straps shall be embedded not less than 12 inches (305 mm) into the chimney. Straps shall be hooked around the outer bars and extend 6 inches (152 mm) beyond the bend. Each strap shall be fastened to not less than four floor joists with two 1/8-inch (12.7 mm) bolts.

R1003.4.1.1 Cold-formed steel framing. Where cold-formed steel framing is used, the location where the 1/8-inch (12.7 mm) bolts are used to attach the straps to the framing shall be reinforced with not less than a 3-inch × 3-inch × 0.229-inch (76 mm × 76 mm × 5.8 mm) steel plate on top of a strap that is screwed to the framing with not fewer than seven No. 6 screws for each bolt.

R1003.5 Corbeling. Masonry chimneys shall not be corbeled more than one-half of the chimney's wall thickness from a wall or foundation, nor shall a chimney be corbeled from a wall or foundation that is less than 12 inches (305 mm) thick unless it projects equally on each side of the wall, except that on the second story of a two-story dwelling, corbeling of chimneys on the exterior of the enclosing walls may equal the wall thickness. The projection of a single course shall not exceed one-half the unit height or one-third of the unit bed depth, whichever is less.

R1003.6 Changes in dimension. The chimney wall or chimney flue lining shall not change in size or shape within 6 inches (152 mm) above or below where the chimney passes through floor components, ceiling components or roof components.

R1003.7 Offsets. Where a masonry chimney is constructed with a fireclay flue liner surrounded by one wythe of masonry, the maximum offset shall be such that the centerline of the flue above the offset does not extend beyond the center of the chimney wall below the offset. Where the chimney offset is supported by masonry below the offset in an approved manner, the maximum offset limitations shall not apply. Each individual corbeled masonry course of the offset shall not exceed the projection limitations specified in Section R1003.5.

R1003.8 Additional load. Chimneys shall not support loads other than their own weight unless they are designed and constructed to support the additional load. Construction of masonry chimneys as part of the masonry walls or reinforced concrete walls of the building shall be permitted.

R1003.9 Termination. Chimneys shall extend not less than 2 feet (610 mm) higher than any portion of a building within 10 feet (3048 mm), but shall be not less than 3 feet (914 mm) above the highest point where the chimney passes through the roof.

R1003.9.1 Chimney caps. Masonry chimneys shall have a concrete, metal or stone cap, a drip edge and a caulked bond break around any flue liners in accordance with ASTM C1283. The concrete, metal or stone cap shall be sloped to shed water.

R1003.9.2 Spark arrestors. All chimneys attached to any appliance or fireplace that burns solid fuel shall be equipped with an approved spark arrester. Where a spark arrester is installed on a masonry chimney, the spark arrester shall meet all of the following requirements:

1. The net free area of the arrester shall be not less than four times the net free area of the outlet of the chimney flue it serves.
2. The arrester screen shall have heat and corrosion resistance equivalent to 12 gage wire, 19-gage galvanized steel or 24-gage stainless steel.
3. Openings shall not permit the passage of spheres having a diameter greater than 1/8 inch (12.7 mm) nor block the passage of spheres having a diameter less than 1/8 inch (9.5 mm).
4. The spark arrester shall be accessible for cleaning and the screen or chimney cap shall be removable to allow for cleaning of the chimney flue.

R1003.9.3 Rain caps. Where a masonry or metal rain cap is installed on a masonry chimney, the net free area under the cap shall be not less than four times the net free area of the outlet of the chimney flue it serves.

R1003.10 Wall thickness. Masonry chimney walls shall be constructed of solid masonry units or hollow masonry units grouted solid with not less than a 4-inch (102 mm) nominal thickness.

R1003.10.1 Masonry veneer chimneys. Where masonry is used to veneer a frame chimney, through-flashing and weep holes shall be installed as required by Section R703.

R1003.11 Flue lining (material). Masonry chimneys shall be lined. The lining material shall be appropriate for the type of appliance connected, in accordance with the terms of the appliance listing and manufacturer's instructions.
ASTM—continued

F628—08 Specification for Acrylonitrile-butadiene-styrene (ABS) Schedule 40 Plastic Drain, Waste and Vent Pipe with a Cellular Core ......................................................... Table P3002.1(1), Table P3002.1(2), Table P3002.2, Table P3002.3, P3003.3, Table AG101.1

F656—10 Specification for Primers for Use in Solvent Cement Joints of Poly (Vinyl Chloride) (PVC) Plastic Pipe and Fittings .......................................................... P2906.9.14, P3003.9.2

F714—13 Specification for Polyethylene (PE) Plastic Pipe (SDR-PR) Based on Outside Diameter .......................................................... Table P3002.2, P3010.4

F876—13 Specification for Cross-linked Polyethylene (PEX) Tubing ......................................................... Table M2101.1, Table P2906.4, Table P2906.5, Table AG101.1

F877—11A Specification for Cross-linked Polyethylene (PEX) Plastic Hot- and Cold-water Distribution Systems ......................................................... Table M2101.1, Table P2906.4, Table P2906.5, Table P2906.6

F891—10 Specification for Coextruded Poly (Vinyl Chloride) (PVC) Plastic Pipe with a Cellular Core ......................................................... Table P3002.1(1), Table P3002.1(2), Table P3002.2, Table P3302.1, Table AG101.1

F1055—13 Specification for Electrofusion Type Polyethylene Fittings for Outside Diameter Controlled Polyethylene and Crosslinked Polyethylene Pipe and Tubing ......................................................... Table M2105.5, M2105.11.2

F1281—11 Specification for Cross-linked Polyethylene/Aluminum/Cross-linked Polyethylene (PEX-AL-PEX) Pressure Pipe ......................................................... Table M2101.1, Table P2906.4, Table P2906.5, Table P2906.6, P2506.11.1, Table AG101.1

F1282—10 Specification for Polyethylene/Aluminum/Polyethylene (PE-AL-PE) Composite Pressure Pipe ......................................................... Table M2101.1, Table P2906.4, Table P2906.5, Table P2906.6, P2906.11.1, Table AG101.1

F1412—09 Specification for Polyolefin Pipe and Fittings for Corrosive Waste Drainage ......................................................... Table P3002.1(2), Table P3002.2, Table P3002.3, P3003.11.1

F1448—09e1 Specification for Coextruded Composite Pipe ......................................................... Table P3002.1(1), Table P3002.1(2), Table P3002.2, Table P3009.11

F1554—07a Specification for Anchor Bolts, Steel, 136, 55 and 105 ksi Yield Strength ......................................................... R608.5.2.2

F1667—11A el Specification for Driven Fasteners, Nails, Spikes and Staples ......................................................... R703.15.1, R703.15.2, R905.2.5

F1807—13 Specification for Metal Insert Fittings Utilizing a Copper Crimp Ring for SDR9 Cross-linked Polyethylene (PEX) Tubing and SDR9 Polyethylene of Raised Temperature (PE-RT) Tubing ......................................................... Table M2101.1, Table P2906.6

F1866—07 Specification for Poly (Vinyl Chloride) (PVC) Plastic Schedule 40 Drainage and DWV Fabricated Fittings ......................................................... Table P3002.3

F1924—12 Standard Specification for Plastic Mechanical Fittings for Use on Outside Diameter Controlled Polyethylene Gas Distribution Pipe and Tubing ......................................................... M2105.11.1

F1960—12 Specification for Cold Expansion Fittings with PEX Reinforcing Rings for Use with Cross-linked Polyethylene (PEX) Tubing ......................................................... Table M2101.1, Table P2906.6

F1970—12 Standard Specification for Special Engineered Fittings, Appurtenances or Valves for Use in Poly (Vinyl Chloride) (PVC) or Chlorinated Poly (Vinyl Chloride) (CPVC) Systems ......................................................... M2105.5, Table 2903.9.4


F1974—09 Specification for Metal Insert Fittings for Polyethylene/Aluminum/Polyethylene and Cross-linked Polyethylene/Aluminum/Cross-linked Polyethylene Composite Pressure Pipe ......................................................... P2506.11.1, Table P2906.6

F1986—01 (2011) Multi-layer Pipe Type 2, Compression Joints for Hot and Cold Drinking Water Systems ......................................................... Table P2906.4, Table P2906.5, Table P2906.6

F2080—12 Specification for Cold-expansion Fittings with Metal Compression-sleeves for Cross-linked Polyethylene (PEX) Pipe ......................................................... P2906.6

F2090—10 Specification for Window Fall Prevention Devices—with Emergency Escape (Egress) Release Mechanisms ......................................................... R310.1.1, R312.2.1, R312.2.2, R612.2, R612.3

F2098—08 Standard Specification for Stainless Steel Clamps for Securing SDR9 Cross-linked Polyethylene (PEX) Tubing to Metal Insert and Plastic Insert Fittings ......................................................... Table M2101.1, Table P2906.6

F2159—11 Standard Specification for Plastic Insert Fittings Utilizing a Copper Crimp Ring for SDR9 Cross-linked Polyethylene (PEX) Tubing and SDR9 Polyethylene of Raised Temperature (PE-RT) Tubing ......................................................... Table P2906.6

F2262—09 Standard Specification for Cross-linked Polyethylene/Aluminum/Cross-linked Polyethylene Tubing OD Controlled SDR9 ......................................................... Table P2906.4, Table P2906.5
### ASTM—continued

<table>
<thead>
<tr>
<th>Standard reference number</th>
<th>Title</th>
<th>Referenced in code section number</th>
</tr>
</thead>
<tbody>
<tr>
<td>F2389—10</td>
<td>Standard for Pressure-rated Polypropylene (PP) Piping Systems</td>
<td>Table M2105.12.1, Table P2906.4, Table P2906.5, Table P2906.6, P2906.10.1, Table AG101.1</td>
</tr>
<tr>
<td>F2434—09</td>
<td>Standard Specification for Metal Insert Fittings Utilizing a Copper Crimp Ring for Polyethylene/Aluminum/Cross-linked Polyethylene (PEX-AL-PEX) Tubing</td>
<td>Table P2906.6</td>
</tr>
<tr>
<td>F2623—08</td>
<td>Standard Specification for Polyethylene of Raised Temperature (PE-RT) SDR9 Tubing</td>
<td>Table M2101.1, Table AG101.1</td>
</tr>
<tr>
<td>F2735—09</td>
<td>Standard Specification for Plastic Insert Fittings for SDR9 Cross-linked Polyethylene (PEX) and Polyethylene of Raised Temperature (PE-RT) Tubing</td>
<td>Table M2101.1, Table P2906.6</td>
</tr>
<tr>
<td>F2769—10</td>
<td>Polyethylene or Raised Temperature (PE-RT) Plastic Hot and Cold-Water Tubing and Distribution Systems</td>
<td>Table M2101.1, Table P2906.4, Table P2906.5, Table P2906.6, Table AG101.1</td>
</tr>
<tr>
<td>F2806—10</td>
<td>Standard Specification for Acrylonitrile-butadiene-styrene (ABS) Plastic Pipe (Metric SDR-PR)</td>
<td>Table M2101.1</td>
</tr>
<tr>
<td>F2855—12</td>
<td>Standard Specification for Chlorinated Poly (Vinyl Chloride)/Aluminum/Chlorinated Poly (Vinyl Chloride) (CPVC AL CPVC) Composite Pressure Tubing</td>
<td>Table P2906.4, Table P2906.5, Table AG101.1</td>
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<tr>
<td>F2969—12</td>
<td>Standard Specification for Acrylonitrile-butadiene-styrene (ABS) IPS Dimensioned Pressure Pipe</td>
<td>Table M2101.1</td>
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### AWC
American Wood Council 222 Carocin Circle, Suite 201 Leesburg, VA 20175

<table>
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<th>Title</th>
<th>Referenced in code section number</th>
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<td>AWC STJIR—2015</td>
<td>Span Tables for Joists and Rafters</td>
<td>R502.3, R502.4, R802.5</td>
</tr>
<tr>
<td>AWC WFCM—2015</td>
<td>Wood Frame Construction Manual for One- and Two-family Dwellings</td>
<td>R301.1.1, R301.2.1.1, R602.10.8.2, R608.9.2, Figure R608.9(9), R608.10</td>
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<td>ANSI AWC NDS—2015</td>
<td>National Design Specification (NDS) for Wood Construction— with 2015 NDS Supplement</td>
<td>R403.2.2, R502.2, Table R503.1, R602.3, R608.9.2, Table R703.15.1, Table R703.15.2, R802.2</td>
</tr>
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<td>ANSI AWC PWF—2015</td>
<td>Permanent Wood Foundation Design Specification</td>
<td>R317.3.2, R401.1, R404.2.3</td>
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### AWPA
American Wood Protection Association P.O. Box 361784 Birmingham, AL 35236-1784

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<thead>
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<th>Referenced in code section number</th>
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<tr>
<td>C1—03</td>
<td>All Timber Products—Preservative Treatment by Pressure Processes</td>
<td>R902.2</td>
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<tr>
<td>M4—11</td>
<td>Standard for the Care of Preservative-treated Wood Products</td>
<td>R317.1.1, R318.1.2</td>
</tr>
<tr>
<td>U1—14</td>
<td>USE CATEGORY SYSTEM: User Specification for Treated Wood Except Section 6 Commodity Specification H</td>
<td>R317.1, R402.1.2, R504.3, R703.6.3, R905.7.5, Table R905.8.5, R905.8.6</td>
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### AWS
American Welding Society 550 N. W. LeJeune Road Miami, FL 33126

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<th>Standard reference number</th>
<th>Title</th>
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<td>A5.8M/A5.8—2011</td>
<td>Specifications for Filler Metals for Brazing and Braze Welding</td>
<td>P3003.6.1</td>
</tr>
<tr>
<td>A5.31M/A5.31—2012</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>
OCCUPIED SPACE
Definition .................................................. R202

P
PARAPETS ................................................. R302.2.2, R302.2.3, R606.4.4
PARTICLEBOARD
Floor ....................................................... R503.3
Walls ....................................................... R605
PERMITS ................................................... 1.8.4, R105
PIERS ....................................................... R606.7
Masonry ..................................................... R404.1.9
PLANNING
Building ................................................... Chapter 3
PLANS ...................................................... 1.8.4, R106
PLASTER
Exterior .................................................... R703.7
Interior ..................................................... R702.2
PLENUM
Definition .................................................. R202
PLUMBING
Fixture clearances ....................................... R307
Inspection ................................................ R109.1.2
PLYWOOD
Application ................................................ R703.5
Materials, walls ......................................... R604
PRECAST CONCRETE
Footings .................................................... R403.4
Foundation material .................................... R402.3.1
Foundation walls ........................................ R404.5
PRIVATE
Sewage disposal system ................................ Appendix I
PROTECTION
Against decay and termites ........................... R317, R318
Against radon ............................................ Appendix F
PURLINS .................................................. R802.5.1
PURPOSE .................................................. 1.1.2, R101.3

R
RADON
Map ......................................................... Appendix F
RAFTERS
Grade of lumber ........................................ R802.1.1
Spans ...................................................... R802.5
Tables R802.5.1(1) – R802.5.1(8)
RAMPS ..................................................... R311.8
RESISTANT SIDING MATERIAL (see MATERIALS)
RIDGE BOARD ............................................. R802.3
RODENT PROOFING ..................................... R602.3.4.1
ROOF
Coverings .................................................. R905
Drainage .................................................... R903.4
Flashing ................................................ R703.4, R903.2, R905
Steel framing .......................................... R804
Wood framing .......................................... R802
ROOF-CEILING CONSTRUCTION (see CONSTRUCTION)
Wind uplift .............................................. R802.11, R804.3.8
ROOFING
Built-up .................................................... R905.9
Liquid-applied coating ................................ R905.15
Modified bitumen ....................................... R905.11
Sprayed polyurethane foam ......................... R905.14
Thermoplastic single-ply ............................. R905.13
Thermoset single-ply .................................. R905.12
ROOM
Minimum sizes .......................................... R304

S
SANITATION ............................................... R306
SEISMIC RISK MAP .................................... Figure R301.2(2)
SHAKES
Wood ..................................................... R702.6, R703.6, R905.8
SHINGLE
Asphalt shingles ........................................ R905.2
Metal ...................................................... R905.4
Slate ....................................................... R905.6
Wood ..................................................... R702.6, R703.6, R905.7
SHOWER
Compartment ............................................ R307.2
SIDING
Exterior coverings ...................................... R703
SITE
Address .................................................. R319
Preparation .............................................. R408.5, R504.2, R506.2
SIZE
Of rooms ................................................ R304
SKYLIGHTS ............................................... R308.6
SLATE SHINGLES ........................................ R905.6
SMOKE ALARMS .......................................... R314
SMOKE-DEVELOPED INDEX ......................... R302.9, R302.10
SNOW LOAD MAP ....................................... Figure R301.2(5)
SOIL INVESTIGATIONS ................................ R401
SOLAR ENERGY SYSTEMS ............................ R324
SPANS
Steel (allowable) ...................................... R505.3.2, R804.3.2.1
Wood (allowable) ...................................... R502.3, R802.5
SPRINKLER (see FIRE SPRINKLER SYSTEMS)
STAIRWAYS .............................................. R311.7
STANDARDS ............................................. Chapter 44, Appendix G
STEEL
Fireplace units ......................................... R1001.5.1
Floor construction .................................... R505
INDEX

Roof-ceiling construction .................................. R804
Walls ................................................... R603
STORM SHELTERS ........................................ R323

STUDS
Wood ........................................... R602.2, R602.3
Spacing ........................................ R602.3.1
Steel .............................................. R603.2, R603.3

SUPPORT
Of decks ............................................. R507.1
Of floor joists ..................................... R502.6, R505.3.2
Of masonry chimneys ................................. R1003.2,
R1003.3, R1003.4

SWIMMING POOLS ..................................... R326

TEMPERATURE ISOLINES ................................ Figure R301.2(1)
TERMINATION
Of chimneys ........................................ R1003.9
TERMITES
Infestation probability map ..................... Figure R301.2(6)
Protection ......................................... R318
THICKNESS
Of chimney walls ................................... R1003.10
TIES
Veneer ............................................... R703.8.4
TILE
Shingles (clay and concrete) ................. R905.3
TOWNHOUSE
Definition .......................................... R202
Scope ............................................. 1.1.3, R101.2
Separation ....................................... R302.2
TRUSSES
Steel ............................................... R505.1.3, R804.3.6
Wood .............................................. R502.11, R802.10

UNDERFLOOR
Space .............................................. R408
UNVENTED ATTIC ASSEMBLIES ...................... R806.5

VAPOR RETARDERS ...................... R506.2.3, R702.7
Capillary break ................................ R506.2.3.1
Definition ....................................... R202
VENEEER
Masonry ............................................. R703.7
VENTILATION
Bathroom .......................................... R303.3

Roof ................................................ R806
Under floor ....................................... R408.1

VIOLATIONS
And penalties .................................... R113
VOLATILE ORGANIC COMPOUNDS (voc's)
Pollutant control ................................ R340

W

WALLBOARD
Gypsum ............................................. R702.3
WALLS
Bracing, steel ..................................... R603.9
Bracing, wood ..................................... R602.10
Construction .................................... Chapter 6
Covering ......................................... Chapter 7
Cripple ............................................ R602.9
Deflection ........................................ R301.7
Exterior covering ................................ R703
Finishes ........................................... R302.9, R702
Fireplace .......................................... R1001.5
Foundation ....................................... R404
Insulating concrete form ....................... R608.3, R608.4,
R608.5.3
Steel framing .................................... R603
Structural insulated panels (SIP) ........ R610
Thickness, masonry chimneys ............... R1003.10
Wood framing .................................. R602

WASTE
Construction waste ................................ R334
WATER CLOSET
Clearances for ................................... R306.1
WATER HEATER, ANCHORAGE ................. R301.2.2.3.7
WATERPROOFING
And dampproofing ................................ R406
WIND SPEED MAP ................................. Figure R301.2(4)A
WINDOW ........................................... R609
Fall prevention ................................... R312.2
Opening limiting devices .................... R312.2.2

WOOD
Floor construction ................................ R502
Foundation walls ................................ R404.2
Roof-ceiling construction .................... R802
Shakes ............................................ R905.8
Shingles .......................................... R905.7
Trusses ............................................... R502.11, R802.10
Walls ............................................... R602

Y

YARD
Definition ........................................ R202
HISTORY NOTE APPENDIX
California Residential Code
Title 24, Part 2.5, California Code of Regulations (CCR)

For prior code history, see the History Note Appendix to the California Residential Code, 2013 Triennial Edition, effective January 1, 2014.


2. Errata to correct editorial errors within the preface as well as throughout various chapters in this code. Effective January 1, 2017.

3. Errata to correct editorial errors throughout the code. Effective September 1, 2017.
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</tr>
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<tbody>
<tr>
<td>AISC</td>
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<td>DOJ</td>
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<td>DOL</td>
<td>NSF</td>
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<td>SMACNA</td>
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<tr>
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<td>USC</td>
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<tr>
<td>AWC</td>
<td>GBI</td>
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</tr>
</tbody>
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